

Little Mindil Beach

25 Gilruth Avenue, The Gardens, Northern Territory

Stormwater Management Plan

KTT Investments Pty Ltd

17 June 2021



Document Verification

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EXECUTIVE SUMMARY

ADG Engineers (Aust.) Pty Ltd was engaged by KTT Investments Pty Ltd to prepare a Stormwater Management Plan (SMP) as additional information in support of a Development Application for a proposed mixed-use development located at 25 Gilruth Avenue, The Gardens, Northern Territory.

This report comprises of a stormwater quantity assessment for the proposed development. This assessment is undertaken to confirm no actionable nuisance of peak flow discharges from the site.

The quantity assessment has identified an increase in the total impervious area across the site as a result of the proposed development. One of the implications of an increase in impervious area is that the total volume and peak flow rate of stormwater runoff from the catchment will increase. Although there has been an increase in peak runoff as a result of the proposed development, due to the sites immediate proximity to an open water body, being the Timor Sea, it is not recommended that detention measures will be required.

The quality assessment has identified that the development should include best practice measures for stormwater quality management in accordance with the Water by Design Deemed to Comply Solutions. Bestmanagement practice for stormwater treatment measures will include:

- Option 1
 - 700m² of Bio-filtration basin;
 - · Green roof: and
 - Litter baskets (200s EnviroPod of approved equivalent) placed within each inlet pit.
- Option 2
 - 190m² of Ocean Protect Filterra;
 - · Green roof; and
 - Litter baskets (200s EnviroPod of approved equivalent) placed within each inlet pit.

All relevant standards and guidelines are addressed in the SMP including criteria from the City of Darwin Subdivision and Development Guidelines, QUDM and Water by Design Guidelines as well as the requirement of AS 3500.3.



1 INTRODUCTION

1.1 GENERAL

ADG Engineers (Aust.) Pty Ltd was engaged by KTT Investments Pty Ltd to prepare a Stormwater Management Plan (SMP) as additional information in support of a Development Application for a proposed mixed-use development located at 25 Gilruth Avenue, The Gardens, Northern Territory.

The purpose of this Stormwater Management Plan is to provide advice as to the development proposal as detailed in the Hachem architectural drawings in **Appendix A**. The works described herein are subject to further approvals and cover works required to service the proposed development with regard to stormwater management.

1.2 BACKGROUND INFORMATION

This report was compiled using information from the following sources:

- Dial Before You Dig' (DBYD) As-Constructed information;
- > City of Darwin Council (CoD) As-Constructed information;
- City of Darwin Subdivision and Development Guidelines (2005);
- Architectural drawings by Hachem (Appendix A); and
- Survey Plan prepared by Land Surveys (Appendix B).

1.3 PROPERTY DETAILS

The total site area is approximately 5.13ha and the existing land titles are provided in **Table 1** below.

Table 1– Property Detail

Title	Lot 7651, Town of Darwin
Street Address	25 Gilruth Avenue, The Gardens, Northern Territory
Site Area	5.13ha

Figure 1 displays the locality of the subject site. The site is bound by Gilruth Avenue to the southeast, the Mindil Beach Casino to the north-east, the Timor Sea to the north-west and Burnett Place to the south.



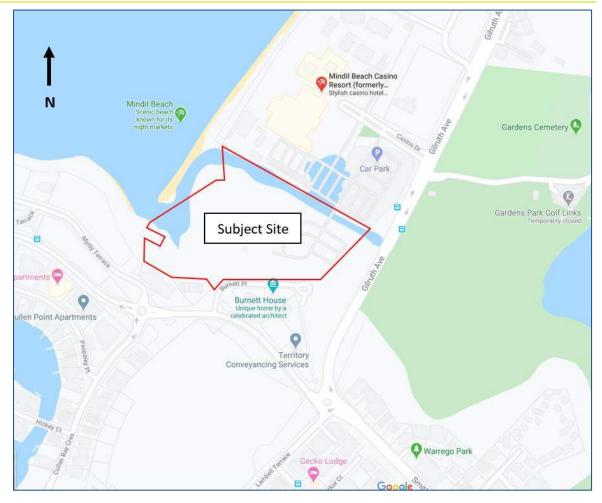


Figure 1 - Locality Map (As accessed from Google maps 30.04.2020)

1.4 Existing Site

The subject site is located at 25 Gilruth Avenue, The Gardens within the City of Darwin local government area and thus the stormwater drainage proposal will be assessed by City of Darwin Council officers. The subject site generally falls from south to north at a grade of approximately 1.00%. The site is bound by an existing 1 in 1 rock escarpment bordering the sites southern boundary.

As it currently stands, the subject site consists of one (1) freehold allotment which is predominantly pervious with an existing sealed carpark occupying the eastern half of the lot. These features account for approximately 30% of the site area. The remaining area may be described as good grass coverage with medium permeability. **Figure 2** illustrates these existing features.

Refer to **Appendix B** for the detailed survey plan prepared by Land Surveys.



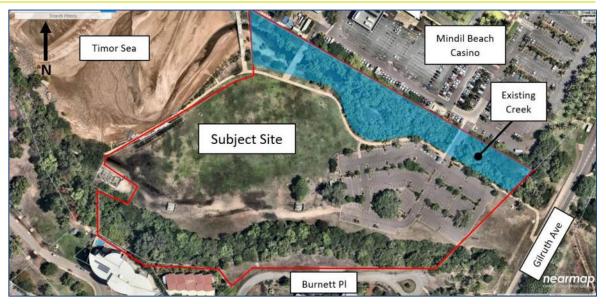


Figure 2 – Existing Site Condition (As Accessed from Nearmap 30.04.2020)



2 PROPOSED DEVELOPMENT

2.1 Development Yield

The proposed development will seek approval for development to create a multi-story hotel, apartment and luxury villa precinct including commercial and function tenancies.

A breakdown of the areas for the proposed development is presented in **Table 2**.

Table 2 - Proposed Development Areas

Land Type	Area (m²)	Percentage of Total Site Area		
Roof Area (Impervious)	18,600	36%		
Ground Area (Impervious)	4,520	9%		
Ground Area (Pervious)	26,140	51%		
Lagoon Area (Impervious)	2,040	4%		
TOTAL	51,300	100%		

Refer to the Architectural drawings prepared by Hachem in **Appendix A** for further information regarding the proposed development.

2.2 Existing Stormwater Infrastructure

Based on information gathered via survey, contour data, aerial imagery and site investigations, it has been determined that the majority of the subject site currently discharges via sheet flows towards the north-eastern boundary. Runoff generated within the site is conveyed toward an existing creek located within the site running parallel to the site's northern boundary. Runoff is then ultimately discharged to the adjacent Timor Sea.

A summary below identifies the existing stormwater infrastructure within the vicinity of the subject site:

- An open-channel natural creek located within the development running parallel to the northern boundary of the site. This drain conveys runoff west to the Timor Sea; and
- The Timor Sea located to the sites north-western boundary.

2.3 Point of Discharge

2.3.1 Existing PD

The existing Lawful Point of Discharge for the site has been identified as the existing creek running parallel to site's northern boundary. ADG note the existing Point of Discharge can be considered tidal as the existing creek within the allotment forms the mouth of the catchment to the Timor Sea.

2.3.2 Proposed PD

The existing Point of Discharge, being the creek running parallel to site's northern boundary, will be maintained for the proposed development. It is proposed to discharge flows to the creek via a



headwall outlet. The proposed outlet will need to consider tidal influence and the affected pipework will need to be saltwater cover class or a material not susceptible to saltwater corrosion (ie Poly Propylene). The proposed outlet should consider the installation of a non-return valve to regulate tidal influence to the internal hydraulic network.

Refer to Appendix C for a preliminary stormwater management layout plan which identifies the location of the Lawful Point of Discharge.

2.4 **External Catchments**

A large external catchment has been identified to discharge to the subject site through the existing creek located within the site parallel to northern boundary. Runoff from this catchment is captured and channelized prior to the subject site and is conveyed within the existing open-channel creek. As such this external catchment does not negatively burden the subject site.

2.5 Flooding Considerations

Refer to the Hydraulic Assessment Prepared by ADG Engineers (Aust.) Pty Ltd for further information on the flooding considerations for the proposed development (Ref: 23085 C R003 dated 28/01/2021).



3 STORMWATER QUANTITY ASSESSMENT

3.1 Proposed Development and Associated Issues

One of the implications of an increase in impervious area is that the total volume and flow rate of stormwater runoff from the catchment will increase. It is essential that these increases are mitigated such that post-developed peak flows and volumes do not exceed those for the pre-developed case. The aim of the stormwater quantity assessment is to:

- Address the need for stormwater quantity control measures.
- Ensure there is no increase in peak discharges from the subject site for events up to and including the 1 in 100 year ARI event.
- Ensure proposed quantity control measures detain and convey flows in accordance with QUDM (2017) minimum freeboard recommendations.

3.2 Flow Rate Methodology

3.2.1 Design Storm Events

Based on recommendations within QUDM 2017, AS/NZ 3500.3 and Council standards the major and initial storm events were selected as follows:

- Initial Event: 1 in 10 year ARI
 - Captured within pit and pipe drainage infrastructure through to Point of Discharge.
- Major Event: 1 in 100 year ARI
 - Surface drainage overflows in events up to and including the 1 in 100 year ARI are to be captured with overland flows paths and road corridors and will not present a hazard to people or cause significant damage to property.

Pipe sizing will be performed during detailed design and increased as required to ensure a safe depth vs velocity is maintained at all times during the major event.

3.2.2 Rational Method for Peak Flow Rate

The peak flow rate for the site has been obtained using the Rational Method in accordance with ARR and QUDM. Summaries of the hydrology calculations can be seen in **Sections 3.3** and **3.4** for the pre and post-development scenarios respectively.

 $Q = (2.78 \times 10^{-3}) \text{ Cy Iy A}$

Equation 1

Q = Peak flow rate (m3/s) for average recurrence interval

Cy = Co-efficient of runoff for ARI of y years (dimensionless)

A = Catchment area (ha)

ly = Average rainfall intensity (mm/hr) for a design duration of t hours and an ARI of y years

3.3 Pre-Development Hydrology

ADG's pre-development stormwater catchment plan has been provided in **Appendix C** for further information on the location of the existing catchment and the corresponding point of discharge.



The hydrology of the pre-developed catchment has been assessed in accordance with QUDM Section 4.0 using the Rational Method. From QUDM Section 4.0, the theoretical calculated peak discharge for storm events ranging from the 1 in 1 year to 1 in 100 year ARIs has been calculated and a summary of the results is presented in **Appendix D**.

3.3.1 Existing Catchment 'A'

Catchment A has an area of 5.13ha and discharges naturally via overland sheet flow over the northern boundary. Since Catchment 'A' is 30% impervious corresponding to a fraction impervious of 0.30, QUDM 2013 (Table 4.5.3) recommends a C₁₀ value of 0.76.

A time of concentration (t_c) of 18.8 minutes has been calculated for the pre-development scenario.

Refer to the Pre-Development Stormwater Catchment Plan in **Appendix C** for further information on the existing catchments.

3.3.2 Rational Method Pre-Development Summary

Table 3 provides a summary of the pre-development peak flow rates calculated using the Rational Method.

Table 3- Pre-development Peak Flow Rates

Catchment	Area	tc	Coefficient of Discharge Peak Flow Rate (m ³ /s)			Peak Flow Rate (m³/s			
	(ha)	(min)	C ₁	C ₁₀	C ₁₀₀	Q1	Q10	Q100	
А	5.19	18.8	0.61	0.76	0.91	0.79	1.64	2.86	

3.4 Post-Development Hydrology

The following sections will assess each catchment in terms of post-development hydrology. The key difference is the increase in impervious areas and shorter time of concentration resulting in an increase to the calculated peak flows.

The hydrology of the pre-development catchments has been assessed in accordance with QUDM Section 4.0 using the Rational Method. From QUDM Section 4.0, the theoretical calculated peak discharge for storm events ranging from the 1 in 1 year to 1 in 100 year ARIs has been calculated and a summary of the results is presented in **Appendix D**.

3.4.1 Proposed Catchment 'C1'

Proposed development Catchment C1 is approximately 49% impervious. A breakdown of the proposed development areas has been provided in Section 2 of this report. QUDM 2013 (Table 4.5.4) recommends a C_{10} value of 0.80 will be required for the corresponding fraction impervious of 0.49.

Due to the high impervious area and the relatively short length of flow for the post-development catchment, a standard inlet time of 10 minutes has been adopted.

Refer to the Post-Development Stormwater Catchment Plan in **Appendix C** for further information on the proposed catchments. Refer to **Appendix C** for a Preliminary Stormwater Management Layout Plan which identifies the location of the discharge point.

3.4.2 Rational Method Post-Development Summary (Unmitigated)

Table 4 provides a summary of the post-development unmitigated peak flow rates calculated using the Rational Method for the catchment.



Table 4– Post-development Peak Flow Rates (Unmitigated)

Catchment	Area	tc	Coeffic	Coefficient of Discharge Peak Flow Rat				
	(ha)	(min)	C₁	C ₁₀	C ₁₀₀	Q1	Q10	Q100
C1	5.19	10	0.64	0.80	0.96	1.05	2.21	3.88

3.5 Recommendation

As demonstrated in **Tables 3** and **4**, there has been an increase in peak flow from the pre to post development unmitigated stage. Although there has been an increase in peak runoff as a result of the proposed development, due to the sites immediate proximity to an open water body, the Timor Sea, it is not recommended that detention measures will be required.

Stormwater drainage infrastructure will be implemented throughout the site to convey stormwater to the proposed point of discharge. A pit and pipe system will be utilised to convey the minor storm flows to the point of discharge. Runoff from a major rainfall event (1 in 100 year) will be conveyed as overland flow through the centralised driveway (ensuring a depth velocity multiplier of less than 0.40) to the point of discharge.

The above proposal is subject to further approvals from City of Darwin, NTG Department of Infrastructure, Planning and Logistics (DIPL) and Department of Environment and Natural Resources (DENR).

Refer to the preliminary stormwater management layout plan in **Appendix C** for further information on the stormwater mitigation scheme.



4 STORMWATER QUALITY ASSESSMENT

This section considers stormwater runoff quality and assesses possible methods of treatment and the subsequent impacts on the drainage strategy. The section determines how to make sure that stormwater leaving the site complies with relevant stormwater quality improvement standards.

This section covers:

- Roof, pavement and hardstand runoff to relevant treatment devices;
- Landscaping runoff;
- Infiltration into the basement; and
- Ensuring treatment device selection criteria are in accordance with Industry Best Practice and, WSUD Engineering Guidelines.

4.1 Site Analysis and Design Strategy

The proposed development and total proposed pervious and impervious areas are summarised in **Table 2** in Section 2 of this report.

Currently no formal stormwater quality management measures are in place for the subject site. The proposed development offers the opportunity to provide stormwater quality treatment where none exists at present.

A MUSIC model analysis was undertaken to determine the extent of the treatment required for the site to achieve the pollutant reduction targets as identified in the NT Subdivision Guidelines. Stormwater treatment measures will include:

Option 1 –

- 700m² of Bio-filtration basin;
- · Green roof; and
- Litter baskets (200s EnviroPod of approved equivalent) placed within each inlet pit.

Option 2 –

- 190m² of Ocean Protect Filterra;
- · Green roof; and
- Litter baskets (200s EnviroPod of approved equivalent) placed within each inlet pit.

Stormwater within the proposed roof areas of the development will be initially treated through the provision of green, landscaped roofs. The internal hydraulic drainage network will direct then flows to 200s EnviroPod litter baskets located within several of the proposed inlet pits prior to discharging to either the bio-filtration system (Option 1) or Filterra (Option 2) located within the landscaping areas of the development and ultimately from the site. Internal hydraulic stormwater drainage shall be designed and constructed in accordance with AS3500.3 and all other relevant standards and guidelines.

The above proposal is subject to further approvals from City of Darwin (CoD) and NTG Department of Environment and Natural Resources (DENR).

For further information on the conceptual layout of the proposed development refer to Architectural drawings in **Appendix A**.



4.2 MUSIC Modelling Results

The sites stormwater run-off was modelled using MUSIC version 6.3.0. The 6-minute rainfall data from 14015 Darwin monitoring site was utilised in the modelling (as recommended by Water by Design Music Modelling Guidelines 2010 and the NT Subdivision Guidelines).

The utilised data was over a 10-year timeframe from 1987 to 1996. Pollutant export parameters for the catchment's different land use types were applied in accordance with Table 3.8 of the above stated guidelines. The objective at each LPD was to achieve the desired target pollutant reduction levels as specified in the NT Subdivision Guidelines dated August 2020. The target pollutant reduction levels were as follows:

- 75% Reduction in Total Suspended Solids (TSS);
- 60% Reduction in Total Phosphorus (TP);
- > 35% Reduction in Total Nitrogen (TN); and
- 90% Reduction in Gross Pollutants.

The following results meet the percent reduction water quality objectives identified by the NT Subdivision Guidelines. Refer to Error! Reference source not found. for further information on the M USIC Model compiled by ADG.

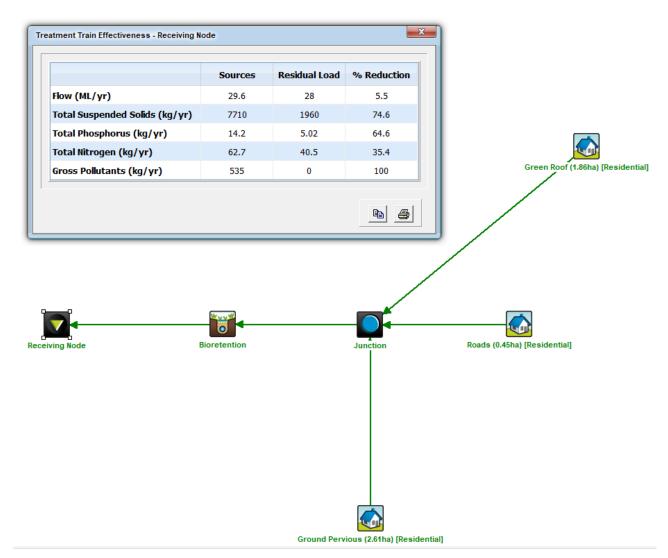
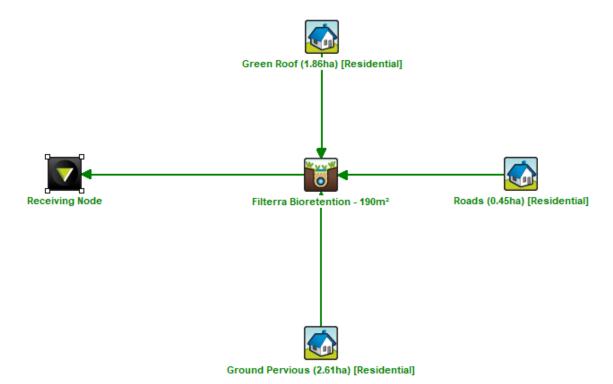


Figure 3 - Treatment Train - Option 1





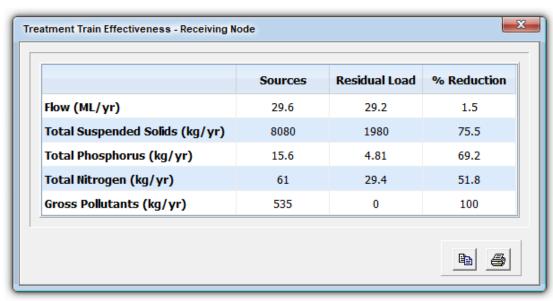


Figure 4– Treatment Train – Option 2

The above results for Options 1 and 2 respectively meet the percent reduction water quality objectives identified by the NT Subdivision Guidelines.

4.3 Operational phase

Once commissioned by the Superintendent and Project Engineers, the Stormwater Quality Improvement Devices (SQIDs) will provide the required level of stormwater quality treatment to runoff from the site prior to discharging into the stormwater drainage infrastructure. It is expected that sediment laden runoff and the erosion potential at the subject site during the operational phase will be minimal. This is due to the high amount of permanent impervious area in the form of roofs, paths,



courtyards, driveways and other impervious structures. The new landscaped areas will be maintained in a manner that will minimise erosion.

4.4 Stormwater Quality Improvement Devices (SQIDs)

Maintenance of the SQIDs will be the responsibility of the Site Manager or Owner, as determined. The maintenance should be carried out in accordance with the manufacturer's recommendations and in line with the maintenance schedule as specified in Section 5.

4.4.1 **EnviroPod**

EnviroPod litter baskets are small baskets consisting of a steel frame and a cage that can be fitted inside standard stormwater inlet pits. Within the cage a screening bag is attached to capture litter, debris, sediment and other pollutants from stormwater flows. The mesh size of the screening bag proposed for each EnviroPod within the site is to be 200 micro-meters. The mesh size is small enough to capture heavy metals and hydrocarbons associated with solids in the stormwater flows.

4.4.2 **Green Roofs**

As identified in the architectural layout plans prepared by Hachem in Appendix A, the development proposal intends to implement green roofs as an aesthetic feature to the net roof area of each building. The green roofs will allow rainwater infiltration which will provide increased water filtration and improve water quality in comparison to a traditional impervious roof. Refer to the Architectural drawings in Appendix A for the location and details of the propsoed green roofs.

4.4.3 **Bio-Filtration Basin**

Bioretention basins remove contaminants and sedimentation from stormwater runoff via infiltration through layers of soil media. The basin is made of a 400mm filter media layer, a 100mm transition layer, and a 300mm drainage layer, located above in-situ soil. The basin is to be vegetated, as per the landscape architect's recommendations. These plants provide additional treatment, and will prevent clogging of the basin. Runoff within the bioretention area gradually infiltrates the soil layers, before discharging through an underdrain to the point of discharge.

4.4.4 **Filterra**

The Ocean Protect Filterra System is a high-flow bio-filtration/retention technology which has been optimised for high volume/flow treatment and high pollutant removal. The Filterra's small footprint allows it to be used on highly developed sites such as landscaped areas, parking lots and streetscapes. Stormwater runoff enters the Filterra System through an inlet and flows through the specially designed filter media prior to discharging to the receiving waterway. The filter media captures and immobilises pollutants; those pollutants are then decomposed, volatilised and incorporated into the biomass of the Filterra system's micro/macro flora and fauna. Stormwater runoff flows through the media and into an underdrain manifold at the bottom of the system.

4.5 Lifecycle Costs

All the recommended water quality treatment infrastructure lies within the development site and it shall be maintained and serviced by the owners of the development at no cost to Council. A lifecycle cost analysis is not part of the scope of this report.

4.6 Water Quality Monitoring

Water quality monitoring is not proposed for this development at this stage due to the nature of the development and the fact that no monitoring currently takes place by another statutory authority.



5 MAINTENANCE

Maintenance of the stormwater network will need to be undertaken regularly by the end user to ensure the system performs as required. **Table 5** outlines a maintenance schedule and associated corrective measures to ensure the stormwater network performs adequately.

Table 5 - Maintenance Schedule

Structure	Maintenance	Maintenance Trigger	Timeframe for Completion of Maintenance			
Grate	Removal of all debris and build-up	The capacity of grated field inlet pit falls below 70%	Prior to the next storm			
Inlet Pit Structure	Removal of all silt and debris build-up in base of pit	The capacity of grated field inlet pit falls below 70%	Prior to the next storm			
Litter Basket	Removal of all silt and debris build-up in litter basket	The capacity of grated field inlet pit falls below 70%	Prior to the next storm			
Stormwater Pipe	Removal of any blockages	The capacity of the stormwater network falls below 70%	Prior to the next storm			
Non-Return Valve	Removal of any blockages including silt and debris build-up	The capacity of the stormwater network falls below 70%	Prior to the next storm			
Bio-filtration Basin	In accordance with the Healthy Waterways guidelines "Guide to the Cost of Maintaining Bioretention Systems" dated February 2015.					
Filterra	In accordance with m	anufacturers maintenance m	anagement plan			



6 Erosion and Sediment Control

6.1 Pre-Development Phase

Prior to construction commencing, the following erosion and sediment control measures will need to be installed around the subject site to minimise disturbance and ensure the quality of runoff discharging from the site is of an acceptable standard:

- Sediment barriers to be installed on all entrances to downstream stormwater infrastructure (i.e. gully pits);
- Designation of transport routes through the site to minimise vegetation disturbance;
- Maximise retention of existing vegetation to reduce soil disturbance and provide filter strip treatment for runoff;
- Install construction entry and exit shakedown areas;
- > Sediment fences are to be installed on the downstream boundaries of the subject site; and
- Install dust control measures as required.

All erosion and sediment control measures are to be designed and installed in accordance with IECA Guidelines. Further details regarding the proposed erosion and sediment control measures will be provided during the detailed design phase of the development.

6.2 Bulk Earthworks Phase

During the bulk earthworks phase, the following erosion and sediment control measure will need to be installed in addition to the aforementioned measures (Pre-Development Phase) to ensure there is minimal disturbance to downstream receiving water bodies:

- Construction chutes to control runoff over earthworks batters:
- Construction of temporary bunds at the top of all earthworks batters to ensure runoff is directed away from exposed batters;
- Sediment basins to be constructed at low points within each stage of the proposed development;
- Construction of temporary diversion drains to divert water to sediment basins and around any stockpiles;
- > Sediment fences to be installed on the downstream side of any stockpiles; and
- Stabilisation of all batters upon reaching the finished earthworks levels.

All erosion and sediment control measures are to be designed and installed in accordance with IECA Guidelines. Further details regarding the proposed erosion and sediment control measures will be provided during the detailed design phase of the development.

6.3 Construction Phase

During the construction phase of the development, there is a risk of sedimentation transport due to large areas of disturbed land. The following erosion and sediment control measure will need to be installed in addition to the aforementioned measures (Pre-Development and Bulk Earthworks



Phases) to ensure there is minimal disturbance and the quality of runoff is maintained to an acceptable standard:

- Construction of temporary diversion drains to divert water to sediment basins;
- Construction of temporary diversion drains to divert water to protect treatment devices as required:
- Sediment barriers to be installed on all entrances to newly constructed stormwater infrastructure (i.e. gully pits);
- Sediment fences to be installed on the downstream side of any stockpiles and batters; and
- Re-vegetation of all disturbed areas within two (2) weeks of completion.

All erosion and sediment control measures are to be designed and installed in accordance with IECA Guidelines. Further details regarding the proposed erosion and sediment control measures will be provided during the detailed design phase of the development.

6.4 Maintenance of ESC Measures

All erosion and sediment control devices are to be maintained through the entire phase of the development leading up to the operational phase. Erosion and sediment control devices will need to be monitored closely throughout the entire project to ensure they are operating correctly and efficiently. No erosion and sediment control devices are to be removed unless otherwise authorised by a suitably qualified engineer or the site superintendent.



7 CONCLUSIONS

Detailed design is to address the above water quantity recommendations. In preparing this report, we have achieved all requirements for Stormwater Management Plans as described in the City of Darwin Subdivision and Development Guidelines, Water by Design Guidelines and QUDM standards, as well as the requirement of AS 3500.3.

Detailed engineering diagrams and management requirements for the proposed development are to be submitted to Council for approval prior to any works commencing on site with design certification prepared by a qualified stormwater engineer or scientist.



Appendix A Architectural Drawings

DRAWING REGISTER ISSUED DATE REV. DATE TP0.00 P11 INFORMATION 22/12/20 22/01/21 DEVELOPMENT SUMMARY & LEGEND TP0.01 P11 INFORMATION 22/12/20 22/01/21 SITE OPPORTUNITIES & CONSTRAINTS TP0.03 P11 INFORMATION 22/12/20 22/01/21 TP0.11 EXISTING SITE CONDITIONS P11 INFORMATION 22/12/20 22/01/21 TP0.12 DEMOLITION PLAN P11 INFORMATION TP0.13 PROPOSED SITE PLAN P11 INFORMATION 22/12/20 22/01/21 TP0.21 ... PERSPECTIVE IMAGESP1.1....INFORMATION... ... 22/01/21 ... 22/01/21... TP1.01 OVERALL PLAN - SEMI-BASEMENT P11 INFORMATION 22/12/20 22/01/21 TP1.02 FLOOR PLAN - SEMI-BASEMENT - HOTEL - PART A P11 INFORMATION 22/12/20 22/01/21 FLOOR PLAN - SEMI-BASEMENT - HOTEL - PART B P11 INFORMATION TP1.03 22/12/20 22/01/21 P11 INFORMATION TP1.04 OVERALL PLAN - GROUND LEVEL 22/12/20 22/01/21 TP1.05 FLOOR PLAN - GROUND LEVEL - SERVICED APARTMENTS / P11 INFORMATION 22/12/20 22/01/21 FORESHORE VILLAS TP1.06 FLOOR PLAN - GROUND LEVEL - HOTEL P11 INFORMATION 22/12/20 22/01/21 FLOOR PLAN - GROUND LEVEL - GARDEN & LAGOON VILLAS - P11 INFORMATION TP1.07 22/12/20 22/01/21 TP1.08 FLOOR PLAN - GROUND LEVEL - LAGOON VILLAS - PART B P11 INFORMATION 22/12/20 22/01/21 TP1.09 OVERALL PLAN - LEVEL 01 P11 INFORMATION 21/12/20 22/01/21 FLOOR PLAN - LEVEL 01 - SERVICED APARTMENTS / ROOF P11 INFORMATION TP1.10 22/12/20 22/01/21 PLAN - FORESHORE VILLAS TP1.11 FLOOR PLAN - LEVEL 01 - HOTEL P11 INFORMATION 22/12/20 22/01/21 TP1.12 ROOF PLAN - GARDEN & LAGOON VILLAS - PART A P11 INFORMATION 12/22/20 TP1.13 ROOF PLAN - LAGOON VILLAS - PART B P11 INFORMATION 22/01/21 12/22/20 TP1.14 OVERALL PLAN - LEVEL 02 P11 INFORMATION . 22/01/21 21/12/20 TP1.15 FLOOR PLAN - LEVEL 02 - SERVICED APARTMENTS P11 INFORMATION22/12/20 P11 INFORMATION TP1.16 FLOOR PLAN - LEVEL 02 - HOTEL 21/12/20 22/01/21 TP1.17 OVERALL PLAN - LEVEL 03 P11 INFORMATION 21/12/20 22/01/21 TP1.18 FLOOR PLAN - LEVEL 03 - SERVICED APARTMENTS P11 INFORMATION 22/12/20 22/01/21 TP1.19 FLOOR PLAN - LEVEL 03 - HOTEL P11 INFORMATION 22/12/20 22/01/21 TP1.20 OVERALL PLAN - LEVEL 04 P11 INFORMATION 21/12/20 22/01/21 TP1.21 FLOOR PLAN - LEVEL 04 - SERVICED APARTMENTS P11 INFORMATION 22/12/20 22/01/21 TP1.22 FLOOR PLAN - LEVEL 04 - HOTEL P11 INFORMATION 22/12/20 22/01/21 TP1.23 P11 INFORMATION 21/12/20 22/01/21 FLOOR PLAN - LEVEL 05 - SERVICED APARTMENTS P11 INFORMATION TP1.24 22/12/20 22/01/21 TP1.25 FLOOR PLAN - LEVEL 05 - HOTEL P11 INFORMATION 22/12/20 22/01/21 TP1.26 OVERALL PLAN - ROOF P11 INFORMATION 22/12/20 22/01/21 TP2.01 OVERALL ELEVATIONS - SERVICED APARTMENT / P11 INFORMATION 14/12/20 22/01/21 FORESHORE VILLAS P11 INFORMATION 14/12/20 22/01/21 TP2.02 OVERALL ELEVATIONS - HOTEL P11 INFORMATION OVERALL ELEVATIONS - GARDEN VILLAS TP2.03 14/12/20 22/01/21 TP2.04 P11 INFORMATION 14/12/20 22/01/21 OVERALL ELEVATIONS - LAGOON VILLAS TP3.02 BUILDING SECTIONS - GARDEN & LAGOON VILLA P11 INFORMATION 01/14/21 22/01/21 TP3.03 BUILDING SECTION - HOTEL / F&B BAR P11 INFORMATION 14/01/20 22/01/21 PROPOSED SHADOW DIAGRAMS

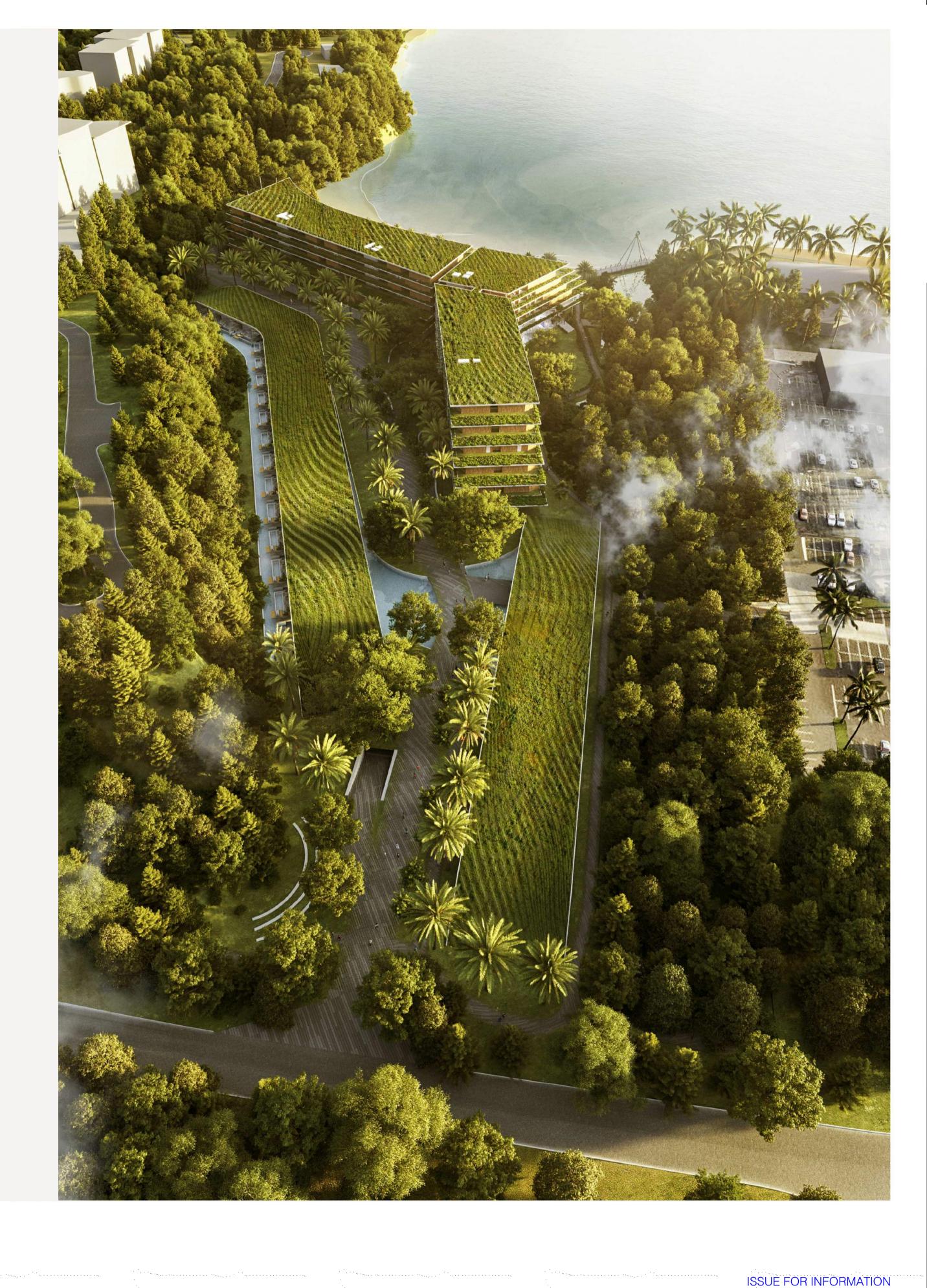
P11 INFORMATION

257

LITTLE MINDIL - 25 GILRUTH AVENUE, THE GARDENS, DARWIN



14/12/20 22/01/21



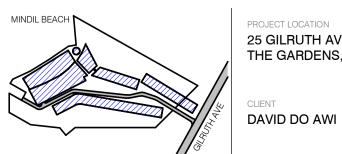
HACHEM ARCHITECTURE INTERIOR MARKETING LEVEL 3, 2 DREWERY PLACE 183 WESTON STREET INFO@HACHEM.COM.AU MELBOURNE 3000 BRUNSWICK EAST 3057 STUDIO +61 1300 734 560 VICTORIA, AUSTRALIA VICTORIA, AUSTRALIA

TP4.01

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P11 ISSUE FOR COORDINATION P10 ISSUE FOR INFORMATION

22/01/21 22/12/20



PROJECT LOCATION DRAWING SCALE SHEET SIZE PROJECT NO 25 GILRUTH AVENUE, THE GARDENS, DARWIN

CHECK DRAWN

KS

DRAWING TITLE **COVER PAGE**

REVISION BUILDING / ATTRIBUTE P11 12°26'59" S 130°49'45" E

PROJECT NAME LITTLE MINDIL

SITE AREA	 Д				ARFA A	NAI YSI	 S	EA AREA ANALYSIS						
NAME		AREA	%	6	DEPARTM				AREA	DEPARTM	ANALYSIS IENT		AREA	
ITE AREA	5	1326 m ²	1	00%	SEMI-BASI HOTEL	EMENT					OOM FLOORS & FUNCTION SPA	CE	3 m ² 586 m ²	
					ADMIN OF				214 m ²	PUBLIC AI	REAS		617 m ²	
EVELOP	MENT	SUMMAR	Υ			PPORT AREA . AREAS & PA			1680 m ² 5496 m ²	RECREAT RESTAUR	IONAL ANT & BARS / SUF	PPORT	461 m ²	
BEDROOM AI			2	2	PUBLIC AF				40 m ²	SERVICES				
BEDROOM G			3		SERVICES VERTICAL	CIRCULATIO	N .		8 m ²	LEVEL 02			5674 m ²	
BEDROOM LA BEDROOM AI			1						7520 m ²	APARTME				
BEDROOM FO			7		GROUND APARTMEI	NT					NT FLOORS		2331 m ²	
BEDROOM AI OTEL ROOM	PARTMEN	T <u>.Ş.</u> . • * *		49		AREAS & PA	ARKING		2767 m ²		IDDODT ADEAG	**		
					SERVICES VERTICAL	CIRCULATIO	 N		297 m ² 38 m ²		JPPORT AREAS DOM FLOORS		109 m² 793 m²	
ROUND BEDROOM G	,	ΙΙ.Δ			HOTEL					PUBLIC AI			1482 m ² 45 m ²	
BEDROOM G			3 1			PPORT AREA	AS		562 m ²	SERVICES)		45 m ²	
BEDROOM F	ORESHOR	E VILLAS	7		EXTERNAL GARDEN V	AREAS & PA	ARKING		811 m ²	LEVEL 03 APARTME	:NIT			
EVEL 01					LAGOON \				13 m ²		NT FLOORS		2266 m ²	
BEDROOM AI BEDROOM AI			6		PUBLIC AF				2408 m ²	HOTEL				
BEDITOON A	FAITHVILIN	10				ANT & BARS ,	/ SUPPORT		105 m ²	BOH & SL	JPPORT AREAS		94 m²	
EVEL 02 BEDROOM AI	PARTMEN	TS	4		RETAIL SERVICES				213 m ²	GUEST RO	OOM FLOORS		2133 m ²	
BEDROOM AI	PARTMEN	TS	1	0		CIRCULATIO	N		13 m ²				4510 m ²	
BEDROOM AI	PARTMEN	IS	1		VILLA					LEVEL 04 APARTME	:NT			
			<u>'</u>		FORESHO				3813 m ²		NT FLOORS		2287 m ²	
VEL 03 BEDROOM AI	PARTMEN	TS	· · · · · · · · · · · 4	• • • • • •	GARDEN \	1.	en e		295 m ²	HÖTEL				
OTEL ROOM.				9	SERVICES				.446 m²	BOH & SL			94 m²	
EVEL 04					LEVEL 01				13471 m²	GUEST ROSERVICES	OOM FLOORS		1956 m ² 17 m ²	
BEDROOM AI			4		APARTME				1600				4354 m ²	
BEDROOM AI BEDROOM AI			1			NT FLOORS PPORT AREA	AS_		1668 m ² 311 m ²	LEVEL 05 APARTME	NT			
OTEL ROOM			4	6	RECREATI	ONAL			399 m²		NT FLOORS		2216 m ²	
VEL 05					SERVICES				111 m ²	HOTEL				
BEDROOM AI	PARTMEN	TS	4		HOTEL	FIOES			16 m²		JPPORT AREAS		94 m ²	
HOTEL ROOM 37			ADMIN OFFICES BOH & SUPPORT AREAS			16 m ² 143 m ²	SERVICES	OOM FLOORS		1812 m ²				
										TOTAL		4140 m²		
										TOTAL			44428 m²	
OTEL RO	OOMS:	SCHEDUL	_E		HOTEL	. ROOMS	SCHED	JLE		APART	MENTS SCI	HEDULE		
OOM NO.	TYPE	ROOM	BALCON	Y TOTAL	ROOM NO	. TYPE	ROOM	··· BALCO	DNY TOTAL	ROOM NO	D. ROOM	BALCONY	····TOTAL	
EVEL 02	. 6		148/214/111	1111000	4012	S4	52 m ²	0 m ²	52 m ²	LEVEL 01				
001 002	S1 S1	68 m ² 70 m ²	13 m ²	80 m ² 82 m ²	4013 4014	K1 K1	38 m ² 38 m ²	0 m ²	38 m ² 38 m ²	101	313 m ² 107 m ²	41 m² 29 m²	354 m ² 135 m ²	
003	S1	72 m ²	13 m ²	85 m²	4015	K1	38 m²	0 m ²	38 m ²	103	104 m²	30 m ²	134 m²	
)04)05	S5 K2	61 m ²	13 m ²	74 m ² 46 m ²	4016 4017	K1 K1	38 m ² 38 m ²	0 m ²	38 m ² 38 m ²	104	104 m ² 103 m ²	30 m ²	134 m ²	
006	S6	69 m ²	22 m ²	90 m²	4018	K1	38 m²	0 m ²	38 m ²	106	104 m²	30 m ²	134 m²	
)07)08	K2 K2	34 m ²	0 m ²	34 m ² 34 m ²	4019 4020	K1 K1	38 m ² 38 m ²	0 m ²	38 m ² 38 m ²	107 108	251 m ² 129 m ²	30 m ²	281 m ²	
009	K2	34 m ²	0 m ²	34 m ²	4021	K1	38 m²	0 m ²	38 m ²	24	1215 m ²	249 m²	1464 m²	
D10 D11	K2 K3	34 m ² 27 m ²	0 m ²	34 m ² 27 m ²	4022 4023	K1 K1	38 m ²	0 m ²	38 m ² 38 m ²	LEVEL 02				
7	110	536 m ²	85 m ²	622 m ²	4024	K1	38 m²	0 m ²	38 m ²	201	166 m²	25 m²	191 m²	
EVEL 03					4025 4026	K1 K1	38 m ² 38 m ²	0 m ²	38 m ² 38 m ²	202	95 m ² 92 m ²	15 m ²	110 m ² 107 m ²	
001	S2	61 m ²	13 m²	74 m²	4027	K1	38 m²	0 m ²	38 m²	204	90 m²	15 m ²	105 m ²	
)02)03	S2 S2	62 m ²	13 m ²	75 m ²	4028 4029	K1 K5	38 m ²	0 m ²	38 m ² 33 m ²	205	106 m ²	30 m ²	136 m ²	
04	· ·S7 · · · · · ·	51 m²	·13 m²···	· · ·63 m²	4030	· · · · · K1 · · · ·	38 m² · ·	· · · 0·m²· ·	· · · · · · 38 m²	207	· · · · · 88 m² · · · · ·	15·m²····	· · · · · 103 m²	
005	K7 K6	32 m ²	13 m ²	44 m ²	4031	K1 K1	38 m ²	0 m ²	38 m ²	208	88 m ²	15 m ²	103 m ²	
07	K2	34 m²	0 m ²	34 m²	4033	K1	38 m²	0 m ²	38 m²	210	90 m ²	15 m²	105 m ²	
08 09	K2 K2	34 m ²	0 m ²	34 m ²	4034 4035	K1 K1	38 m ² 38 m ²	0 m ²	38 m ² 38 m ²	211 212	115 m ²	30 m ²	145 m ² 162 m ²	
10	K2	34 m ²	0 m ²	34 m ²	4036	K1	38 m²	0 m ²	38 m ²	213	109 m ²	15 m²	124 m ²	
11	K3	27 m ²	0 m ²	27 m ²	4037	K1	38 m ²	0 m ²	38 m ²	214	116 m ²	15 m ²	131 m ²	
12 13	K4 S4	39 m ² 51 m ²	0 m ²	39 m ² 51 m ²	4038 4039	K1 K12	38 m ² 38 m ²	0 m ²	38 m ² 38 m ²	215 34	210 m ² 1692 m ²	13 m ² 292 m ²	223 m ² 1984 m ²	
14	K1	38 m ²	0 m ²	38 m²	4040	K1	38 m²	0 m ²	38 m²	LEVELOC				
15 16	K1	38 m ² 38 m ²	0 m ²	38 m ² 38 m ²	46		1525 m ²	81 m ²	1605 m ²	201	157 m ²	25 m ²	182 m ²	
17	K1	38 m ²	0 m ²	38 m²	LEVEL 05	177.	10 1	10 -		202	100 m ²	15 m²	115 m ²	
18 19	K1	38 m ²	0 m ²	38 m ² 38 m ²	5001 5002	K11 K2	42 m ² 36 m ²	13 m ²	55 m ² 49 m ²	203	96 m ² 93 m ²	15 m ²	111 m ² 108 m ²	
20	K1	38 m ²	0 m ²	38 m²	5003	K2	36 m ²	13 m ²	49 m ²	207	91 m²	15 m ²	105 m ²	
)21)22	K1	38 m ²	0 m ²	38 m ²	5004	K2 K2	36 m ²	13 m ²	48 m ²	208	91 m ²	15 m ²	105 m ²	
23	. K1	38 m ²	.0 m² · · · ·	``` 38 m²	5006	S6	45 m²	15 m²	·····60 m²	210	92 m²····	15 m²	107 m²	
24 25	. <u>K1</u>	38.m² 38 m²	.0.m² 0 m²	38 m² 38 m²	5007 5008	K2 K2	34 m² 34 m²	0.m² 0 m²	34 m ²	213 214	109 m ²	14.m² 15 m²	<u>1</u> 23 m² 129 m²	
26	K1	38 m ²	0 m ²	38 m ²	5009	K3	27 m²	0 m ²	27 m ²	215	196 m²	14 m ²	209 m ²	
27 28	K1 K1	38 m ² 38 m ²	0 m ²	38 m ²	5010 5011	K4 S4	39 m ² 51 m ²	0 m ²	39 m ² 51 m ²	301 302	107 m ² 105 m ²	30 m ²	137 m ² 135 m ²	
28 29	K1	38 m ²	0 m ²	38 m ²	5011	K2	38 m ²	0 m ²	38 m ²	302	105 m ² 114 m ²	30 m ²	135 m ² 144 m ²	
30	K5 K1	33 m ² 38 m ²	0 m ²	33 m ²	5013 5014	K2 K2	38 m ² 38 m ²	0 m ²	38 m ²	304	128 m ²	30 m ²	158 m ²	
31 32	K1	38 m ²	0 m ²	38 m ² 38 m ²	5014	K2	38 m ²	0 m ²	38 m ² 38 m ²	. 34	1685 m²	292 m ²	1977 m²	
)33	K1	38 m ²	0 m ²	38 m ²	5016	K2	38 m²	0 m ²	38 m ²	LEVEL 04	450 0	04 0	474 ^	
)34)35	K1	38 m ² 38 m ²	0 m ²	38 m ² 38 m ²	5017 5018	K2 K2	38 m ² 38 m ²	0 m ²	38 m ² 38 m ²	401 402	150 m ² 103 m ²	24 m ² 15 m ²	174 m ² 118 m ²	
36	K1	38 m ²	0 m ²	38 m²	5019	K2	38 m²	0 m ²	38 m²	403	98 m²	15 m²	113 m²	
)37)38	K1	38 m ²	0 m ²	38 m ² 38 m ²	5020 5021	K2 K2	38 m ²	0 m ²	38 m ²	404 405	93 m ²	15 m ²	108 m ²	
39	K1	37 m ²	0 m ²	37 m ²	5022	K2	38 m²	0 m ²	38 m ²	406	105 m ²	30 m ²	135 m²	
)40)41	K1	38 m ²	0 m ²	38 m ²	5023	K2	38 m ²	0 m ²	38 m ²	407	88 m ²	15 m ²	103 m ²	
141 <u></u> 142	K1 K12	38 m ² 38 m ²	0 m ²	38 m ² 38 m ²	5024 5025	K2 K2	38 m ² 38 m ²	0 m ²	38 m ² 38 m ²	408	88 m ² 88 m ²	15 m ²	103 m ²	
43	·K1	38 m²	.0.m²	38 m²	···· 5026 · · · · ·	K2		0.m²	38 m²	410	88 m². · · · · ·	· · · · · 15 m² · · · · ·	103 m²	
		1682 m²	83 m ²	1766 m²	5027 5028	K2 K5	38 m ² 33 m ²	0 m ²	38 m ² 33 m ²	411 412	109 m ²	30 m ²	138 m ² 148 m ²	
VEL 04					5029	K2	38 m ²	0 m ²	38 m ²	413	100 m ²	15 m ²	115 m ²	
01	S3	52 m ²	13 m ²	64 m ²	5030	K2	38 m ²		38 m²	414	104 m ²	15 m ²	119 m ²	

38 m²

38 m²

1390 m² 78 m²

5133 m² 327 m² 5461 m²

38 m²

38 m²

1468 m²

5034

5035

5036 5037

TOTAL

34 m²

27 m²

VICTORIA, AUSTRALIA

PARKING SUMMARY LOCATION GROUND BICYCLE SPACES SEMI-BASEMENT HOTEL AND FUNCTION CAR SPACES GROUND APARTMENT CAR SPACES HOTEL AND FUNCTION CAR SPACES HOTEL VILLA BUGGY SPACES ON-GRADE CAR SPACES PORT-COCHERE CAR SPACES PORT-COCHERE BUGGY SPACES CAR PARKING TOTAL

	M09	The state of the s
	CONCRETE PAVER	
TOTAL		
105 m ²		
105 m ²		
105 m ²		
112 m²		

277

APARTMENTS SCHEDULE

90 m²

100 m²

180 m²

112 m²

106 m²

112 m²

1634 m²

7861 m²

BALCONY

 $15 \, \mathrm{m}^2$

15 m²

15 m²

15 m²

15 m²

16 m²

30 m²

30 m²

30 m²

30 m²

293 m²

1418 m²

115 m²

195 m²

142 m²

135 m²

136 m²

142 m²

1927 m²

9279 m²

ROOM NO. ROOM

409

410

414

503

15 m²

292 m²

15 m²

209 m²

1928 m²

120 m²

114 m²

194 m²

105 m²

99 m²

1636 m²

TOTAL

EXTERNAL FINISHES

TIMBER BOARD FORMED

CONCRETE

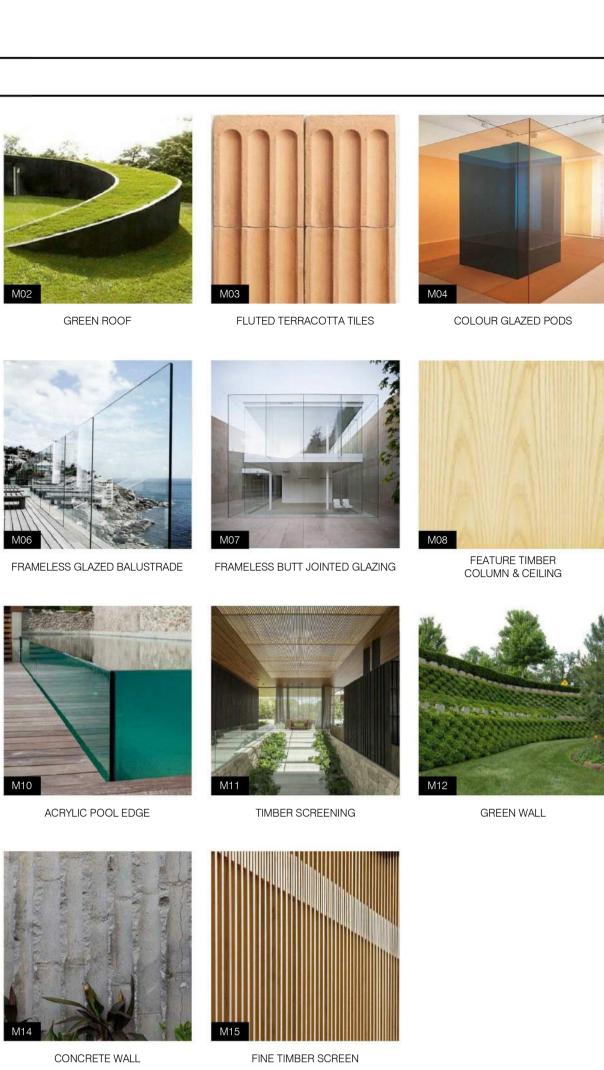
TIMBER MULLION GLAZING





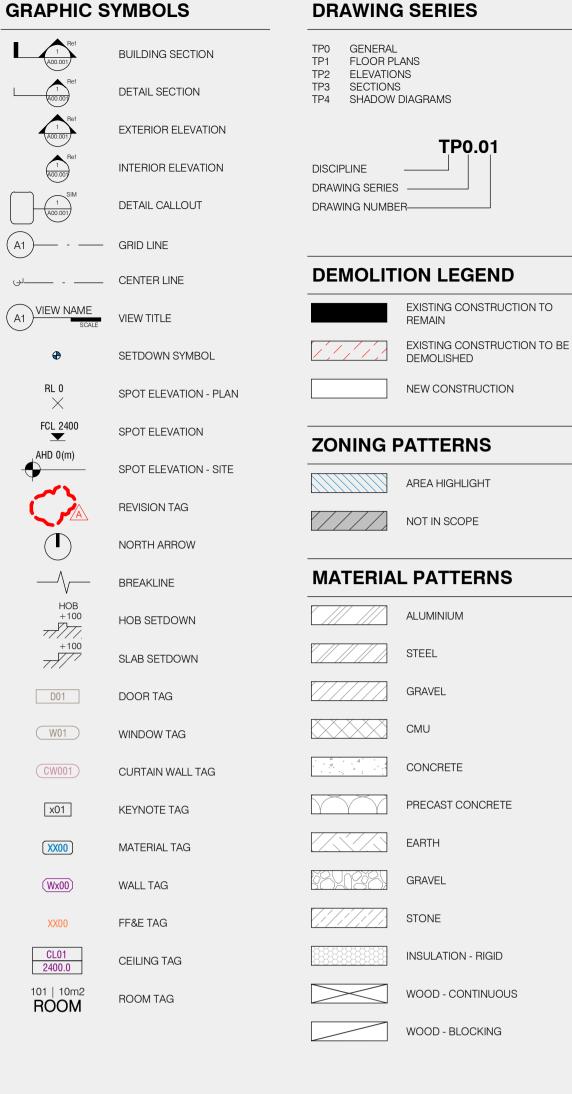
ACRYLIC POOL EDGE

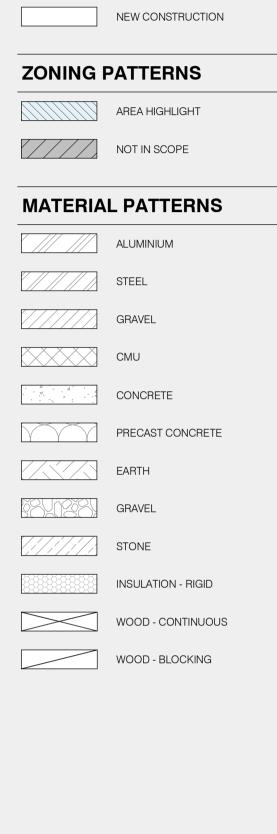
GREEN ROOF





WIR WALK-IN ROBE





HACHEM ARCHITECTURE INTERIOR MARKETING

0 m²

39 m² 0 m² 39 m²

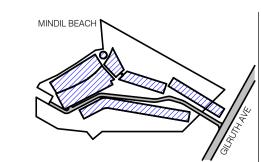
27 m²

4009

LEVEL 3, 2 DREWERY PLACE 183 WESTON STREET INFO@HACHEM.COM.AU MELBOURNE 3000 BRUNSWICK EAST 3057 STUDIO +61 1300 734 560 VICTORIA, AUSTRALIA

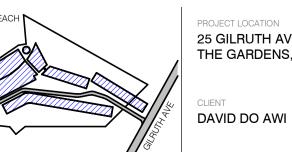
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P11 ISSUE FOR COORDINATION P10 ISSUE FOR INFORMATION



22/01/21

22/12/20



PROJECT LOCATION 25 GILRUTH AVENUE, THE GARDENS, DARWIN

DRAWING DATE CHECK DRAWN 22/12/20 EP

DRAWING SCALE SHEET SIZE PROJECT NO A1 257 KS



DRAWING TITLE DEVELOPMENT SUMMARY & LEGEND

PROJECT NAME

LITTLE MINDIL

P11 12°26'59" S 130°49'45" E

ISSUE FOR INFORMATION

REVISION BUILDING / ATTRIBUTE

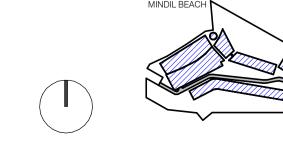


HACHEM ARCHITECTURE INTERIOR MARKETING

HACHEM.COM.AU LEVEL 3, 2 DREWERY PLACE 183 WESTON STREET INFO@HACHEM.COM.AU MELBOURNE 3000 BRUNSWICK EAST 3057 STUDIO +61 1300 734 560 VICTORIA, AUSTRALIA VICTORIA, AUSTRALIA

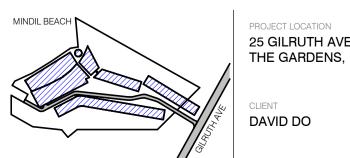
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REV. DESCRIPTION P10 ISSUE FOR INFORMATION P9 ISSUE FOR INFORMATION



15/01/21

22/12/20



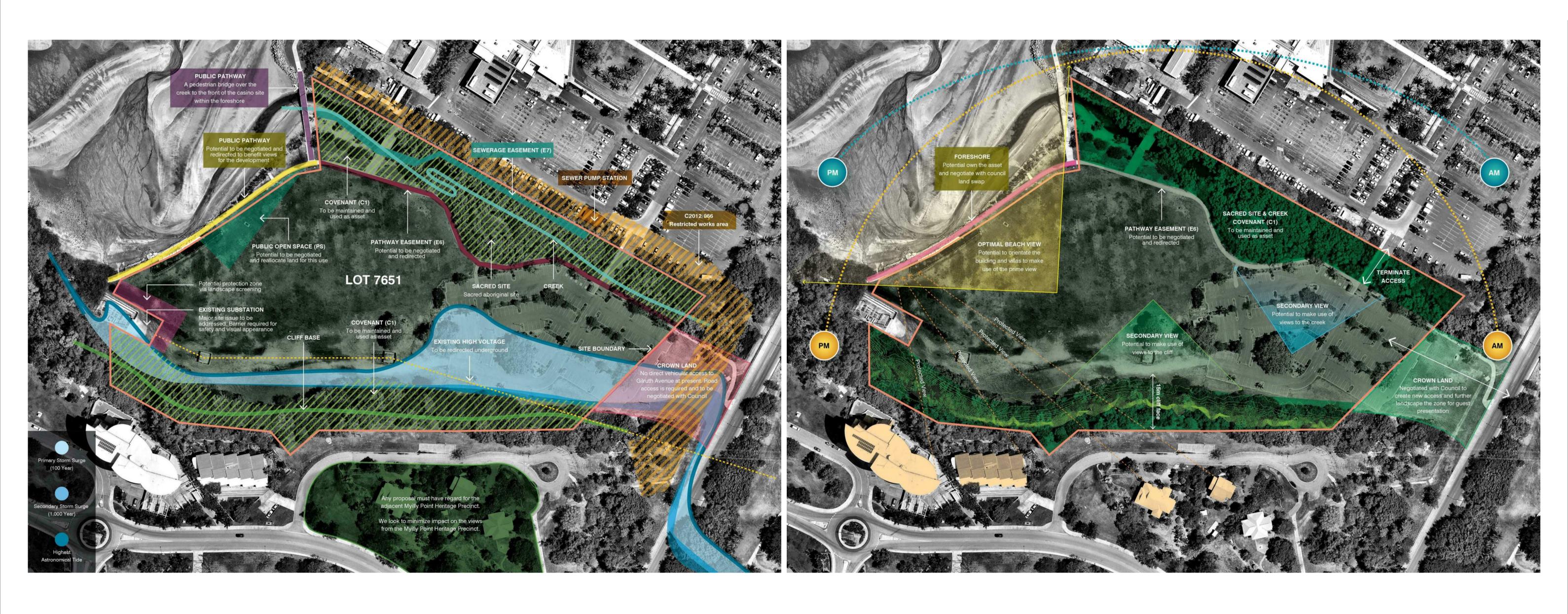
DRAWING SCALE SHEET SIZE PROJECT NO A1 257 25 GILRUTH AVENUE, THE GARDENS, DARWIN

DRAWING TITLE DESIGN RESPONSE WITH SITE PHOTOS

P10 12°26'59" S 130°49'45" E

ISSUE FOR INFORMATION

REVISION BUILDING / ATTRIBUTE



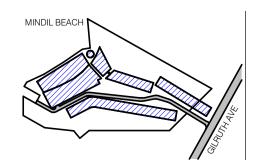
HACHEM ARCHITECTURE INTERIOR MARKETING LEVEL 3, 2 DREWERY PLACE 183 WESTON STREET

INFO@HACHEM.COM.AU MELBOURNE 3000 BRUNSWICK EAST 3057 STUDIO +61 1300 734 560 VICTORIA, AUSTRALIA VICTORIA, AUSTRALIA

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REV. DESCRIPTION P11 ISSUE FOR COORDINATION P10 ISSUE FOR INFORMATION

22/01/21 22/12/20



PROJECT LOCATION 25 GILRUTH AVENUE, THE GARDENS, DARWIN 22/12/20 EP KS DAVID DO AWI

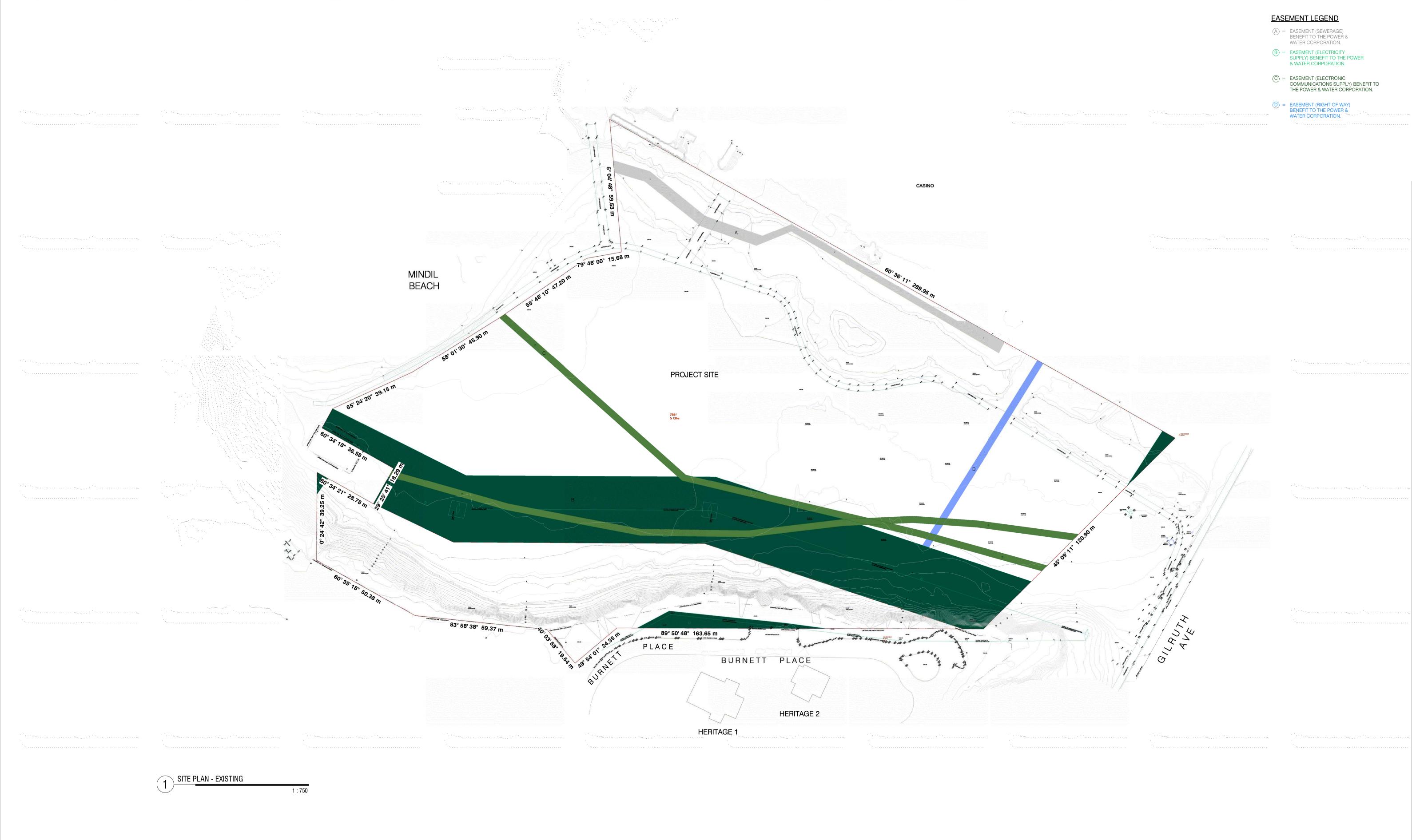
DRAWING SCALE SHEET SIZE PROJECT NO A1 257 DRAWING DATE CHECK DRAWN

DRAWING TITLE SITE OPPORTUNITIES & CONSTRAINTS PROJECT NAME

LITTLE MINDIL

ISSUE FOR INFORMATION REVISION BUILDING / ATTRIBUTE P11 12°26'59" S 130°49'45" E

TP0.03



DATE

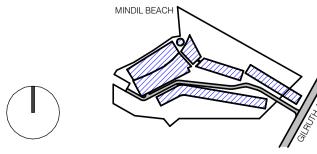
22/01/21

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HACHEM ARCHITECTURE INTERIOR MARKETING HACHEM.COM.AU LEVEL 3, 2 DREWERY PLACE 183 WESTON STREET INFO@HACHEM.COM.AU MELBOURNE 3000 BRUNSWICK EAST 3057 STUDIO +61 1300 734 560 VICTORIA, AUSTRALIA VICTORIA, AUSTRALIA

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REV. DESCRIPTION P11 ISSUE FOR COORDINATION P10 ISSUE FOR INFORMATION



PROJECT LOCATION THE GARDENS, DARWIN DAVID DO AWI

DRAWING SCALE SHEET SIZE PROJECT NO 25 GILRUTH AVENUE, As indicated A1 257 DRAWING DATE CHECK DRAWN
22/12/20 EP KS

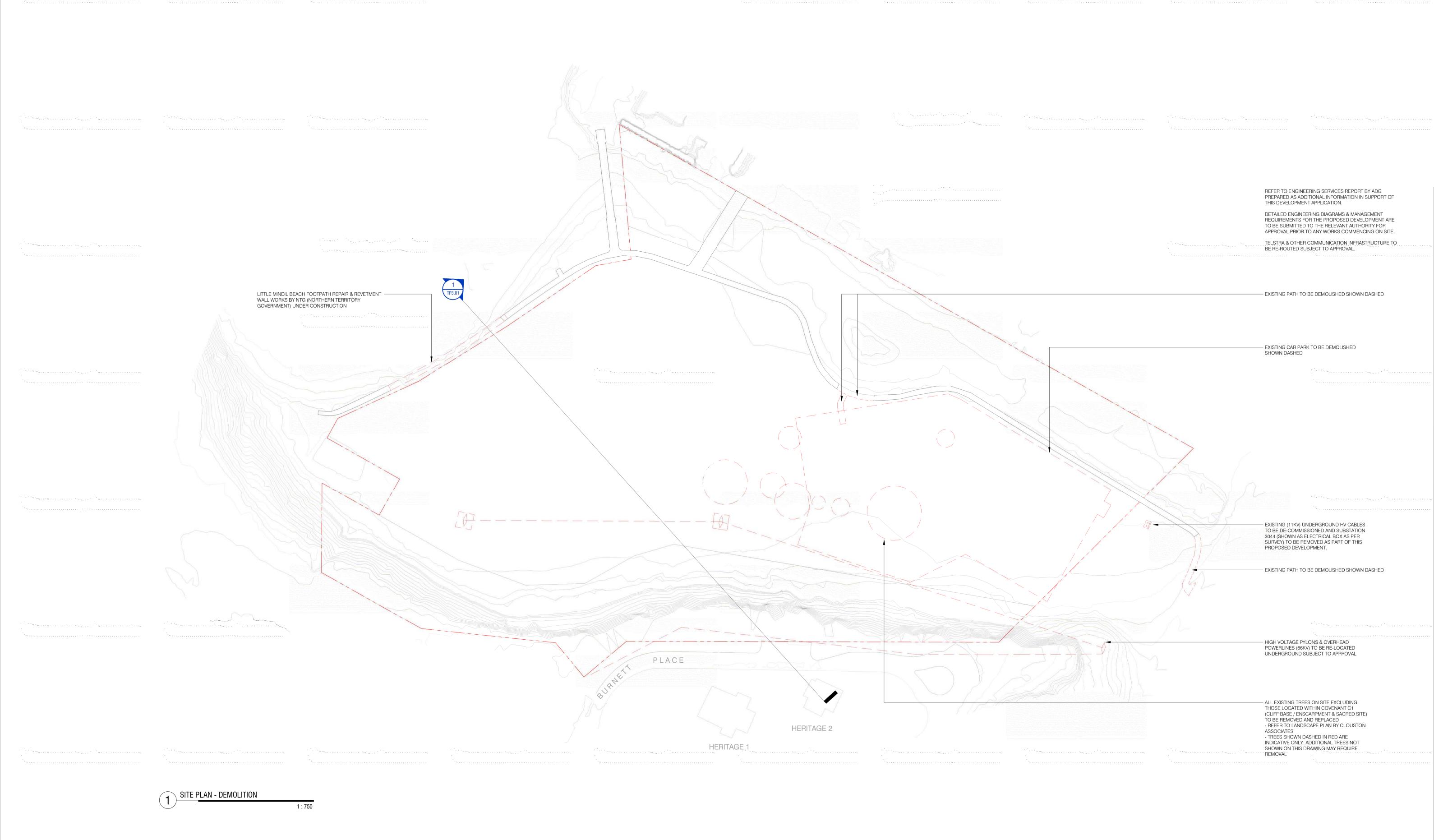
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DRAWING TITLE EXISTING SITE CONDITIONS

PROJECT NAME LITTLE MINDIL

REVISION BUILDING / ATTRIBUTE P11 12°26'59" S 130°49'45" E

ISSUE FOR INFORMATION



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22/01/21

22/12/20

PROJECT LOCATION THE GARDENS, DARWIN

DRAWING SCALE SHEET SIZE PROJECT NO 25 GILRUTH AVENUE, 1:750 A1 257

KS

DRAWING TITLE **DEMOLITION PLAN**

PROJECT NAME

LITTLE MINDIL

REVISION BUILDING / ATTRIBUTE P11 12°26'59" S 130°49'45" E

ISSUE FOR INFORMATION

scale drawings. Figured dimensions must be used in preference to scaled dimensions. Notify Architect of any discrepancies.



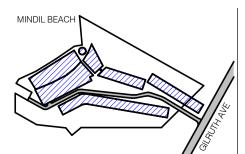


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22/12/20



PROJECT LOCATION DRAWING SCALE SHEET SIZE PROJECT NO 25 GILRUTH AVENUE, 1:750 A1 257 THE GARDENS, DARWIN DRAWING DATE CHECK DRAWN EP KS DAVID DO AWI 22/12/20

DRAWING TITLE PROPOSED SITE PLAN

> PROJECT NAME LITTLE MINDIL

REVISION BUILDING / ATTRIBUTE P11 12°26'59" S 130°49'45" E

ISSUE FOR INFORMATION







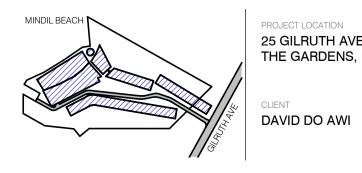
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REV. DESCRIPTION P11 ISSUE FOR COORDINATION

22/01/21



PROJECT LOCATION 25 GILRUTH AVENUE, THE GARDENS, DARWIN

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22/01/21 EP KS

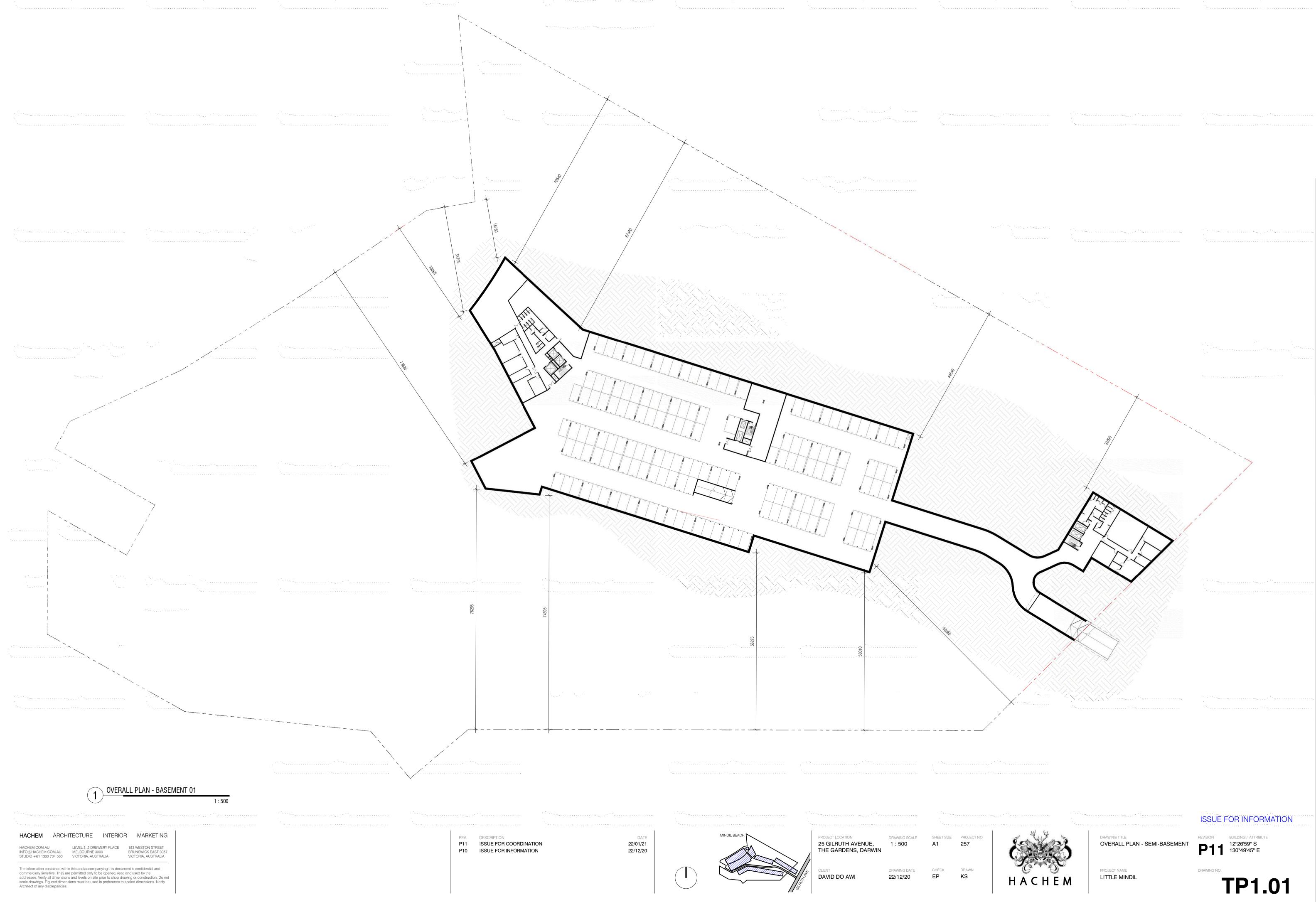
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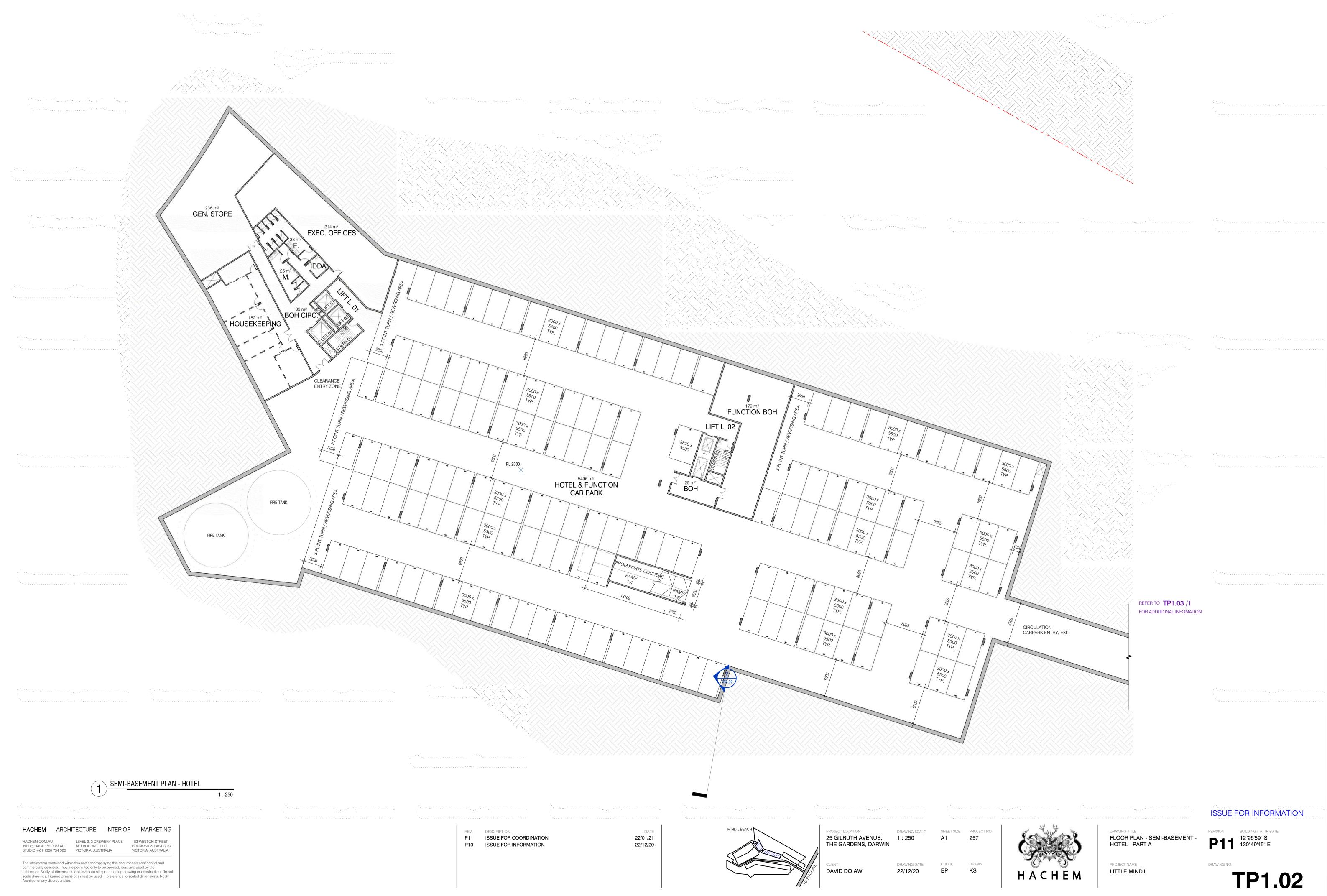
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PROJECT NAME

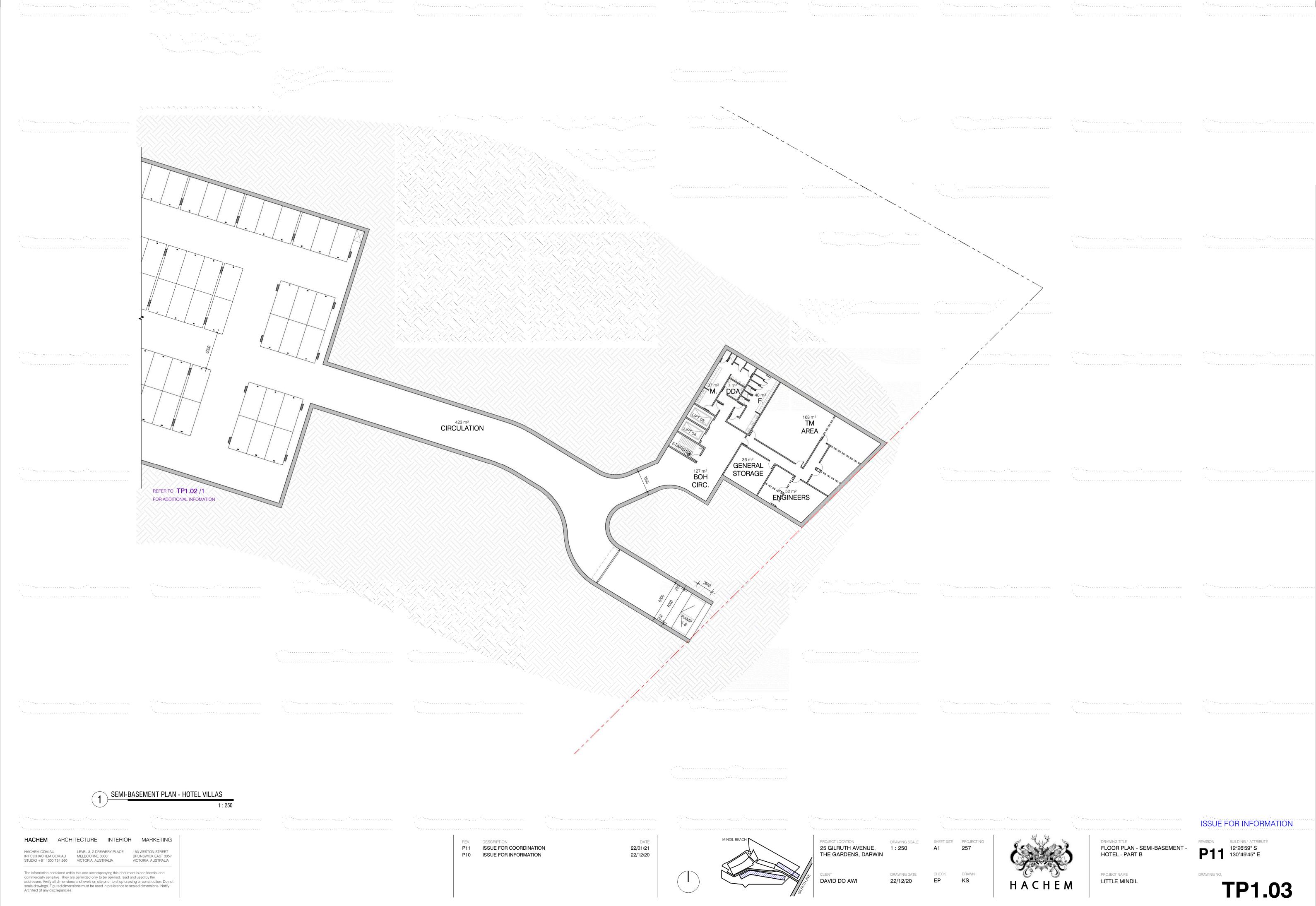
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ISSUE FOR INFORMATION

BUILDING / ATTRIBUTE
12°26'59" S
130°49'45" E

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ISSUE FOR INFORMATION

REVISION BUILDING / ATTRIBUTE P11 12°26'59" S 130°49'45" E

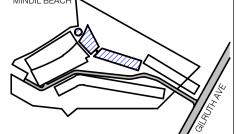
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REV. DESCRIPTION P11 ISSUE FOR COORDINATION P10 ISSUE FOR INFORMATION

22/01/21 22/12/20



PROJECT LOCATION DRAWING SCALE SHEET SIZE PROJECT NO 25 GILRUTH AVENUE, 1: 250 A1 257 THE GARDENS, DARWIN DRAWING DATE CHECK DRAWN EP KS DAVID DO AWI 22/12/20

PROJECT NAME LITTLE MINDIL

HOTEL

FLOOR PLAN - GROUND LEVEL -



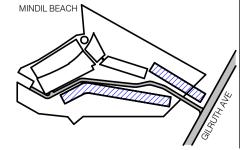
GROUND PLAN - HOTEL VILLAS - PART A

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PROJECT LOCATION DRAWING SCALE SHEET SIZE PROJECT NO 25 GILRUTH AVENUE, 1:250 A1 257 THE GARDENS, DARWIN DRAWING DATE CHECK DRAWN DAVID DO AWI EP KS 22/12/20



DRAWING TITLE FLOOR PLAN - GROUND LEVEL -GARDEN & LAGOON VILLAS - PART PROJECT NAME LITTLE MINDIL

REVISION BUILDING / ATTRIBUTE P11 12°26'59" S 130°49'45" E

TP1.07

ISSUE FOR INFORMATION



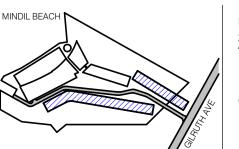
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HACHEM ARCHITECTURE INTERIOR MARKETING

REV. DESCRIPTION P11 ISSUE FOR COORDINATION P10 ISSUE FOR INFORMATION

DATE 22/01/21 22/12/20



PROJECT LOCATION THE GARDENS, DARWIN DAVID DO AWI

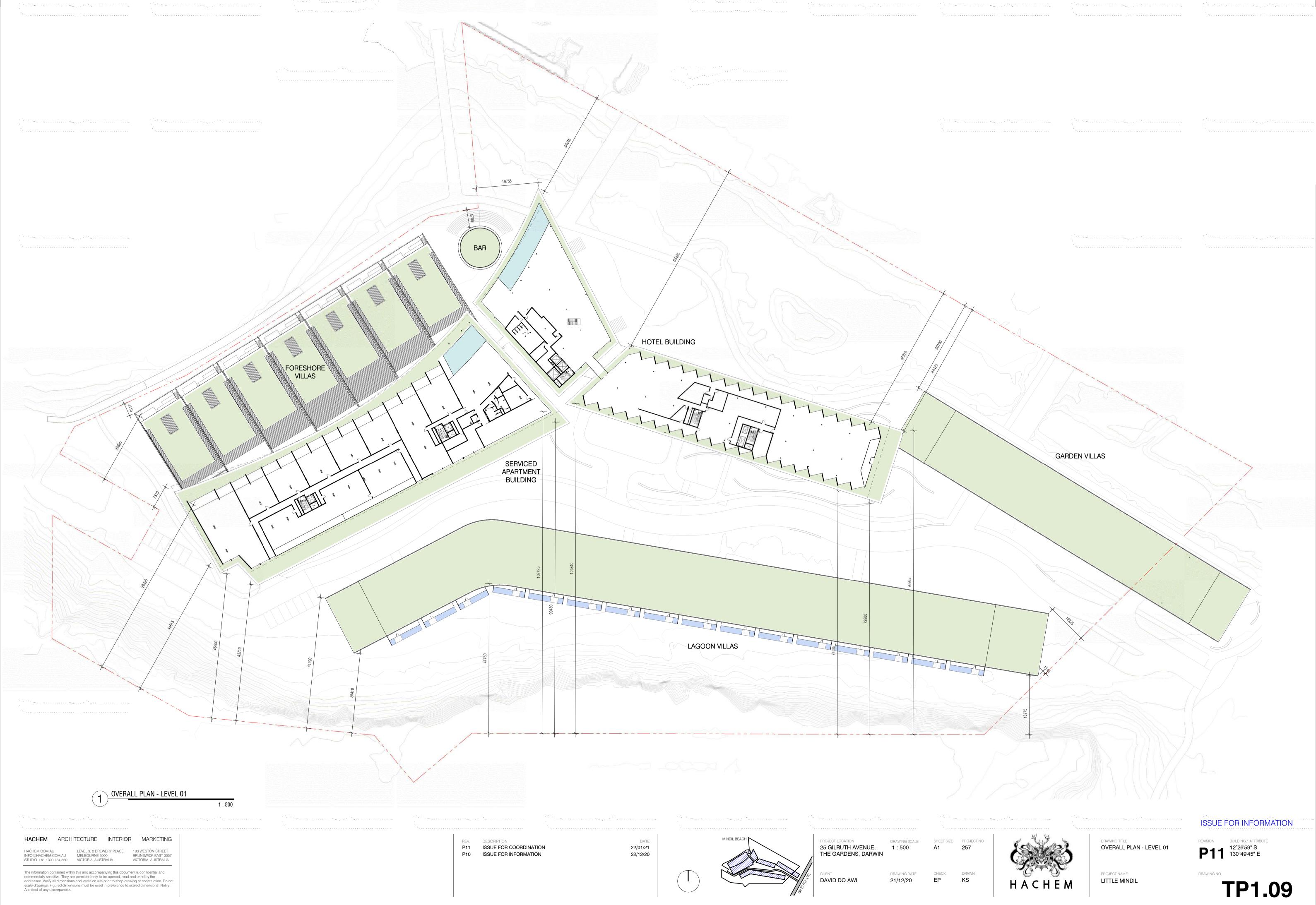
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PROJECT NAME LITTLE MINDIL

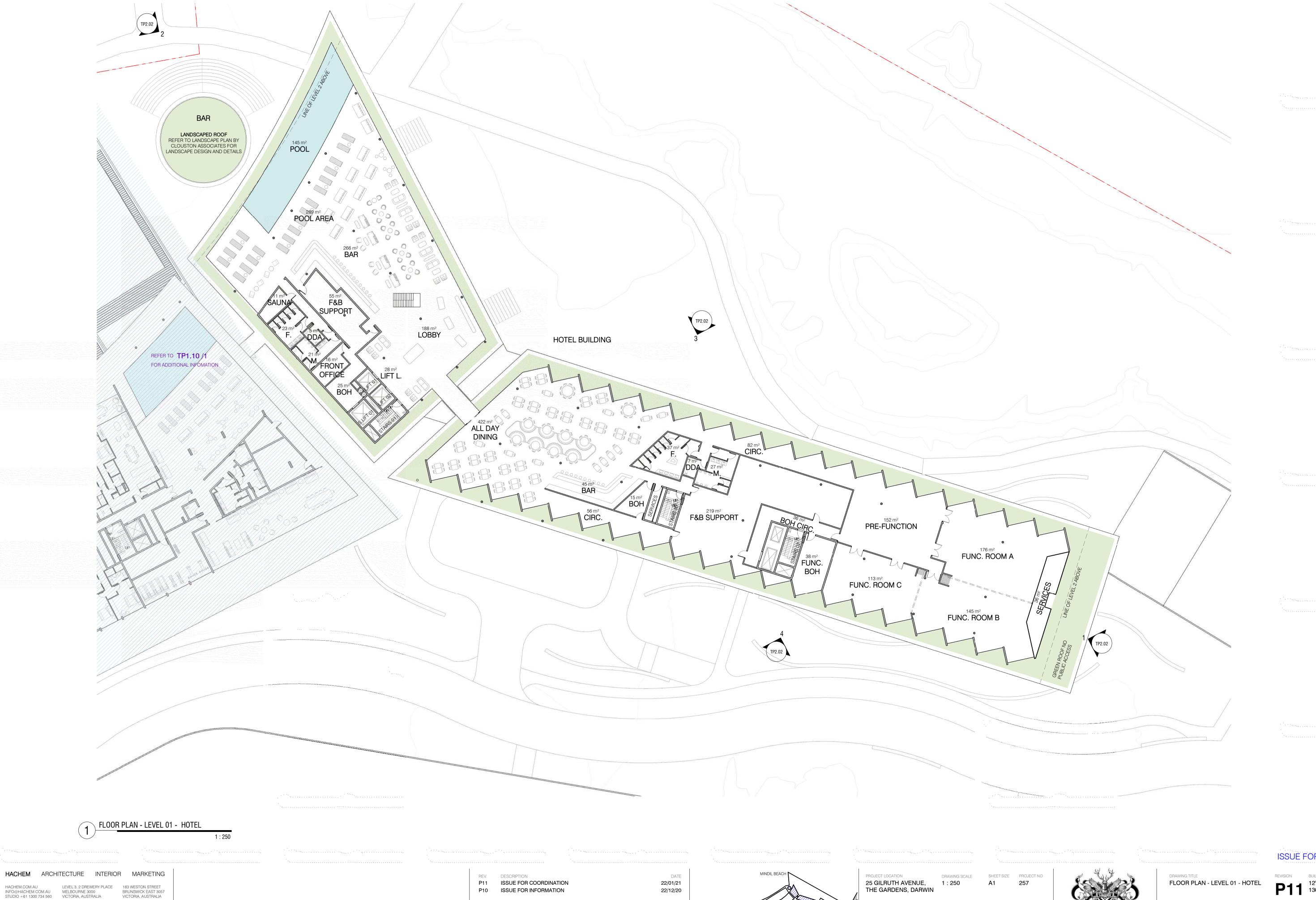
ISSUE FOR INFORMATION REVISION BUILDING / ATTRIBUTE

P11 12°26'59" S 130°49'45" E



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REVISION BUILDING / ATTRIBUTE P11 12°26'59" S 130°49'45" E

PROJECT NAME

LITTLE MINDIL

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22/12/20 EP KS

DAVID DO AWI

TP1.11

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DAVID DO AWI

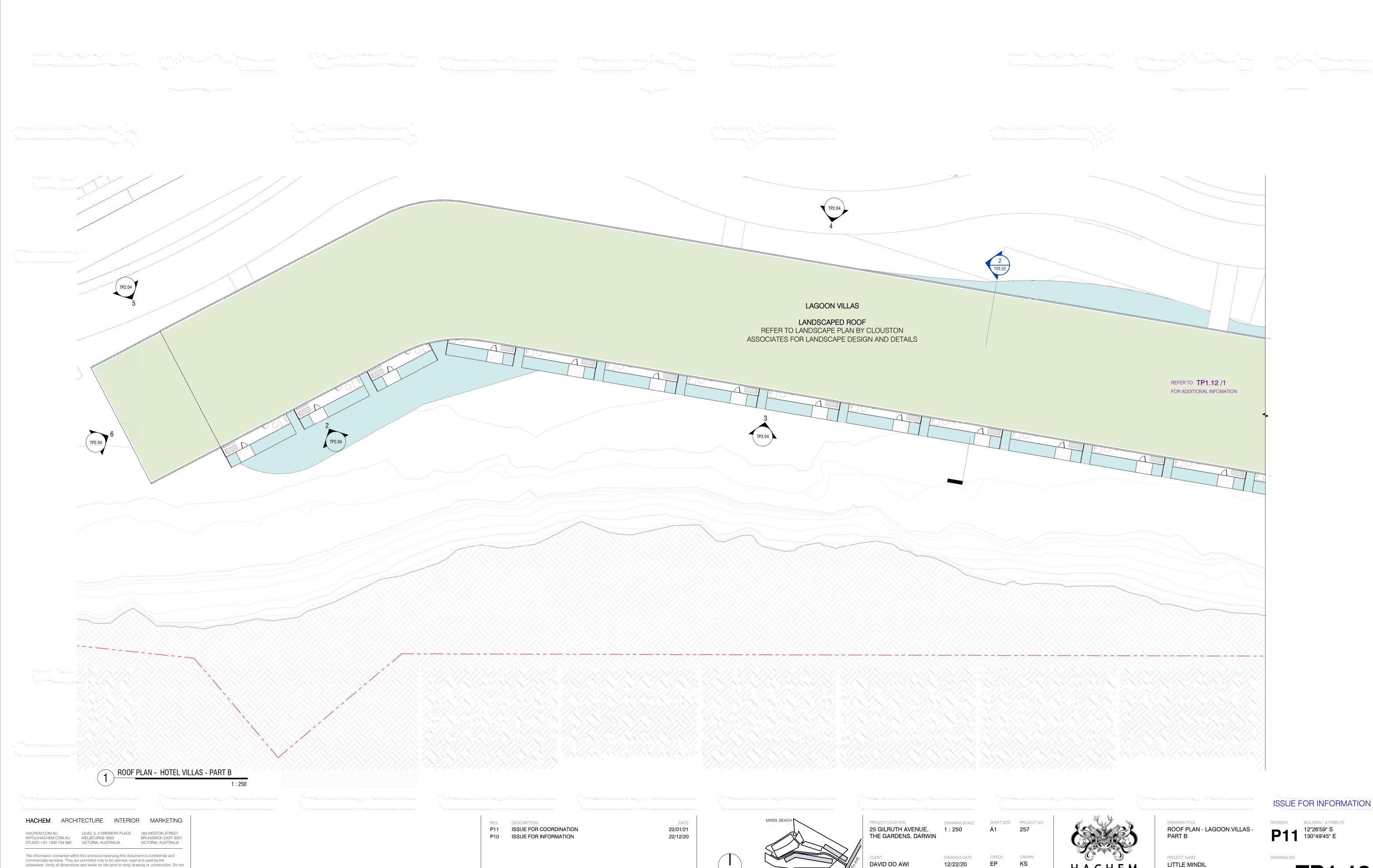
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PROJECT NAME

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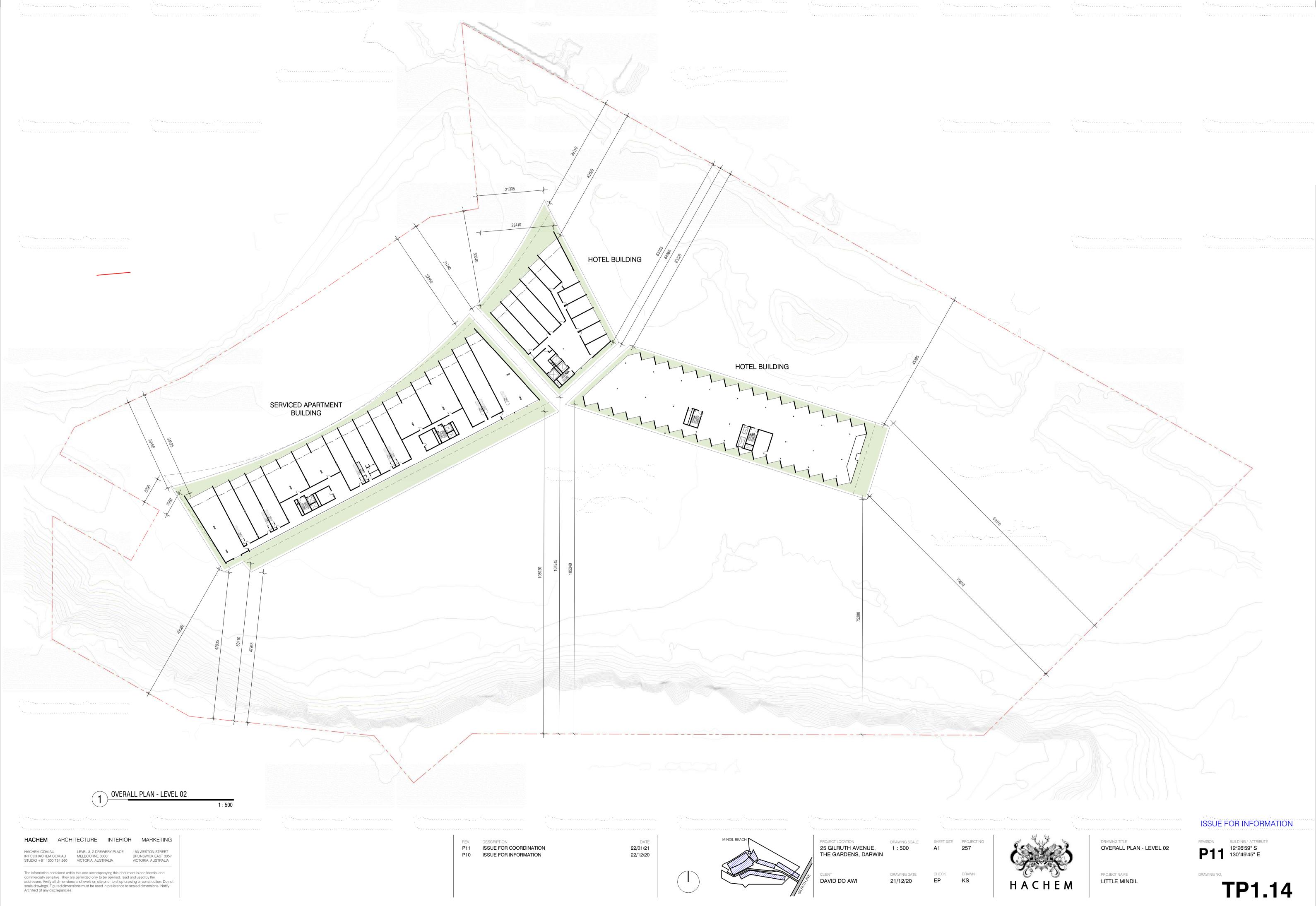
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LITTLE MINDIL **TP1.13**

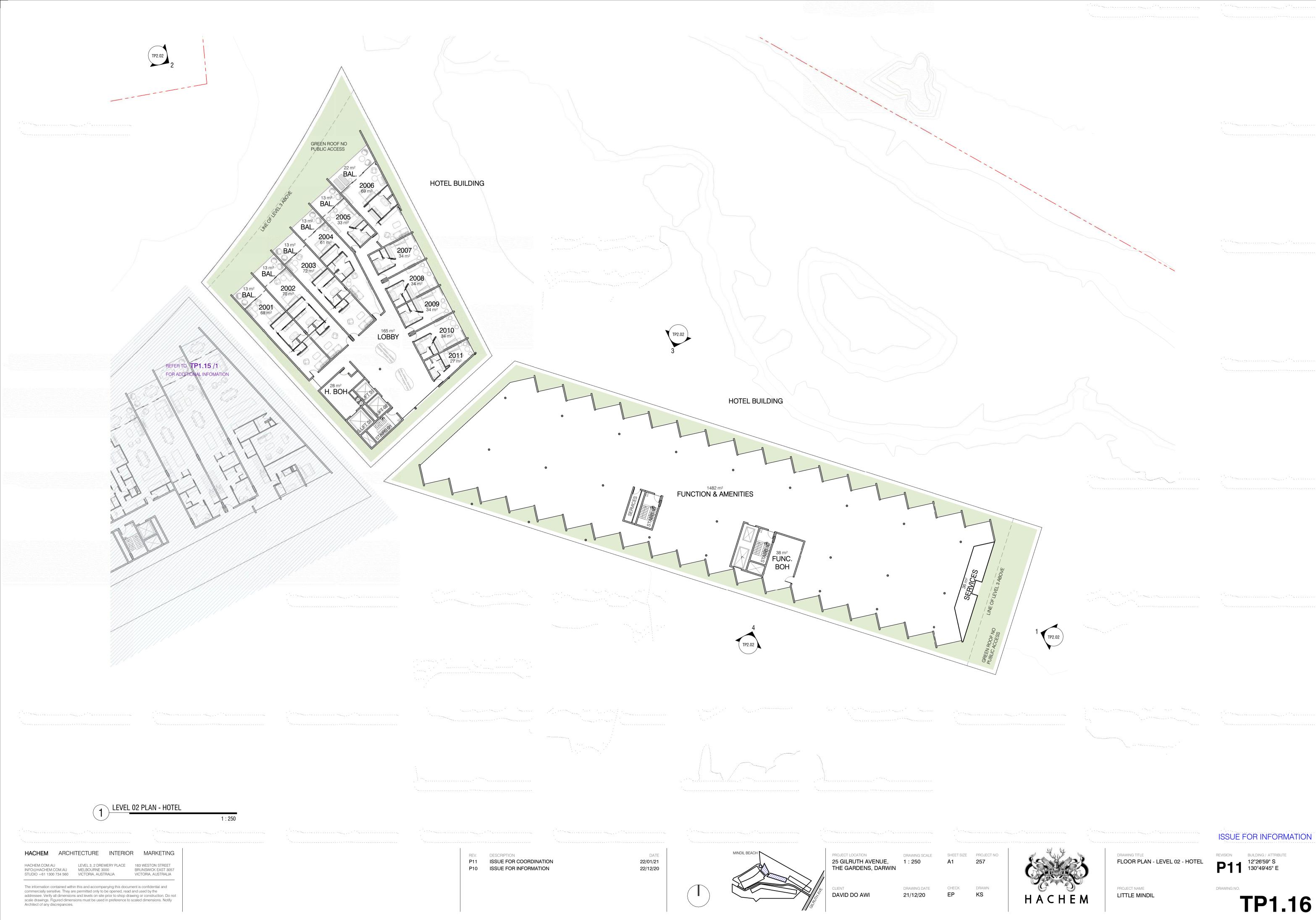
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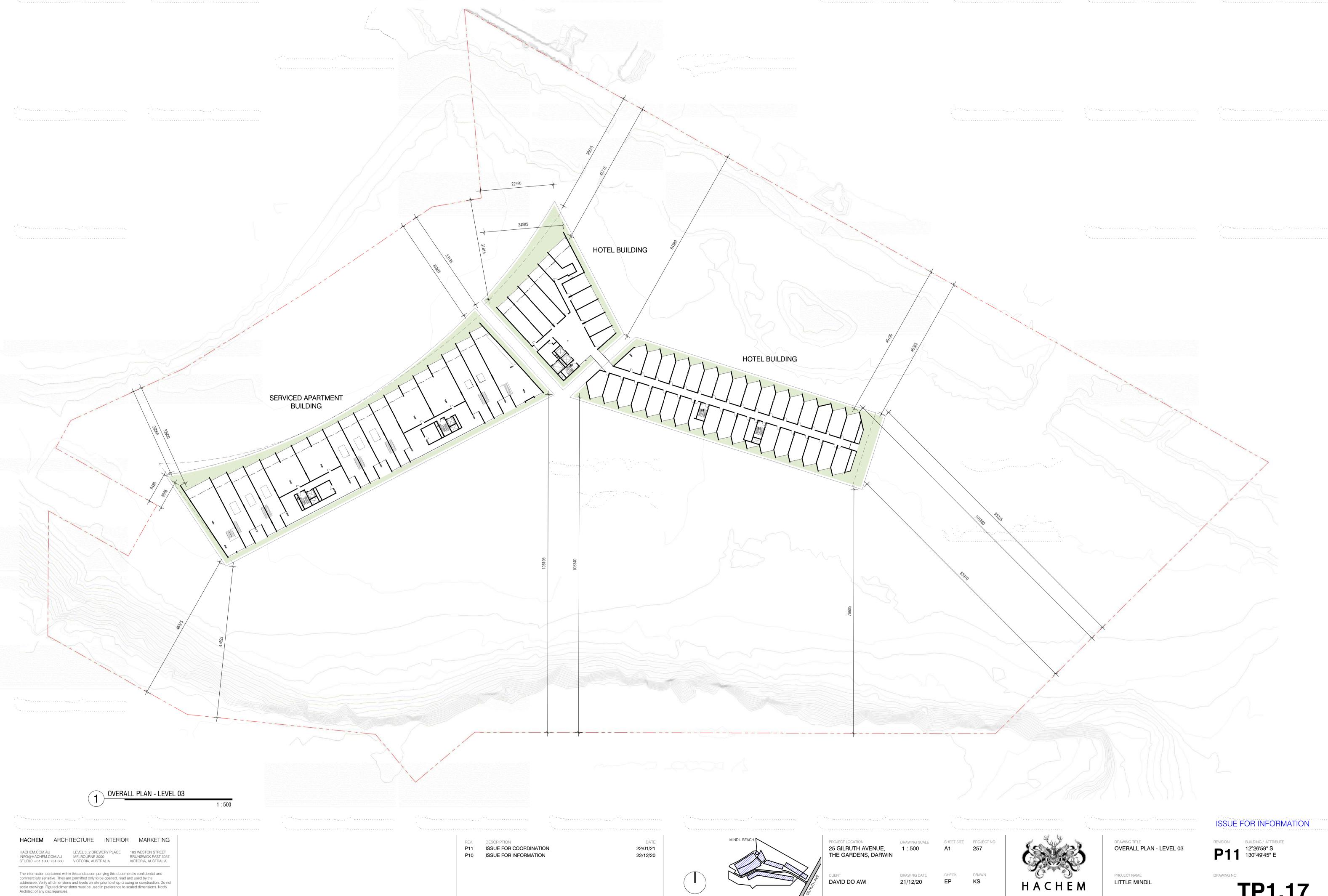


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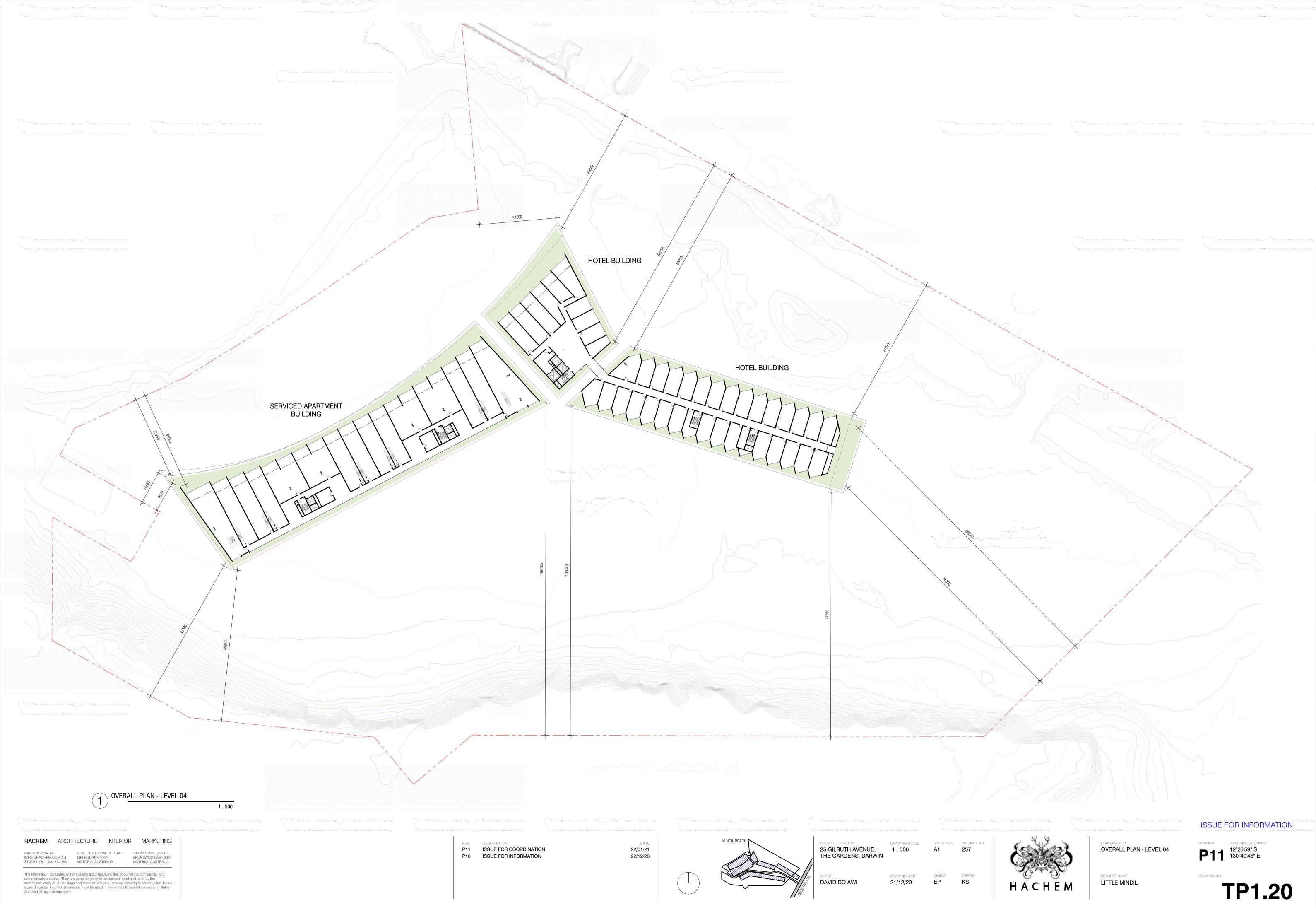
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P10 ISSUE FOR INFORMATION

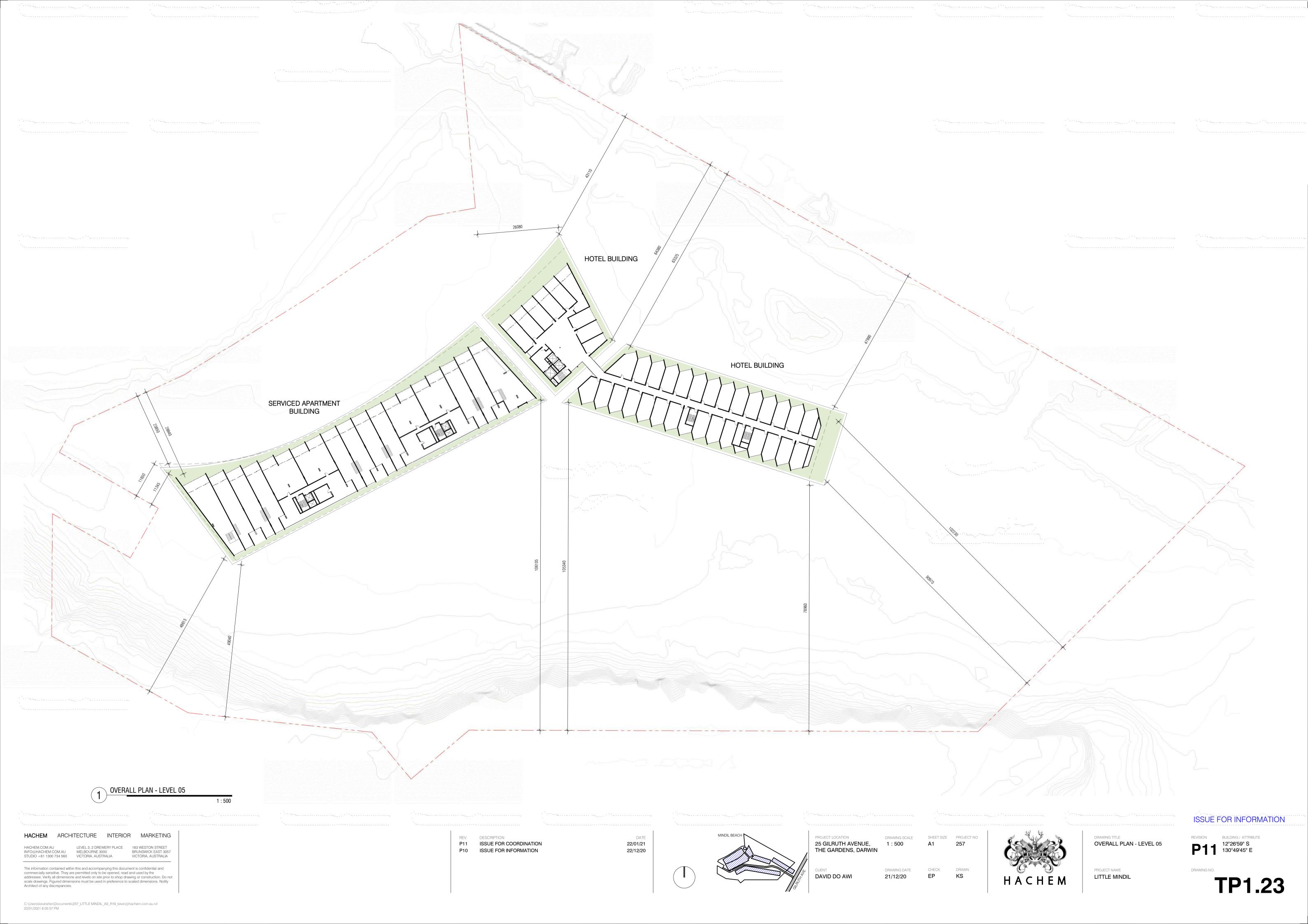
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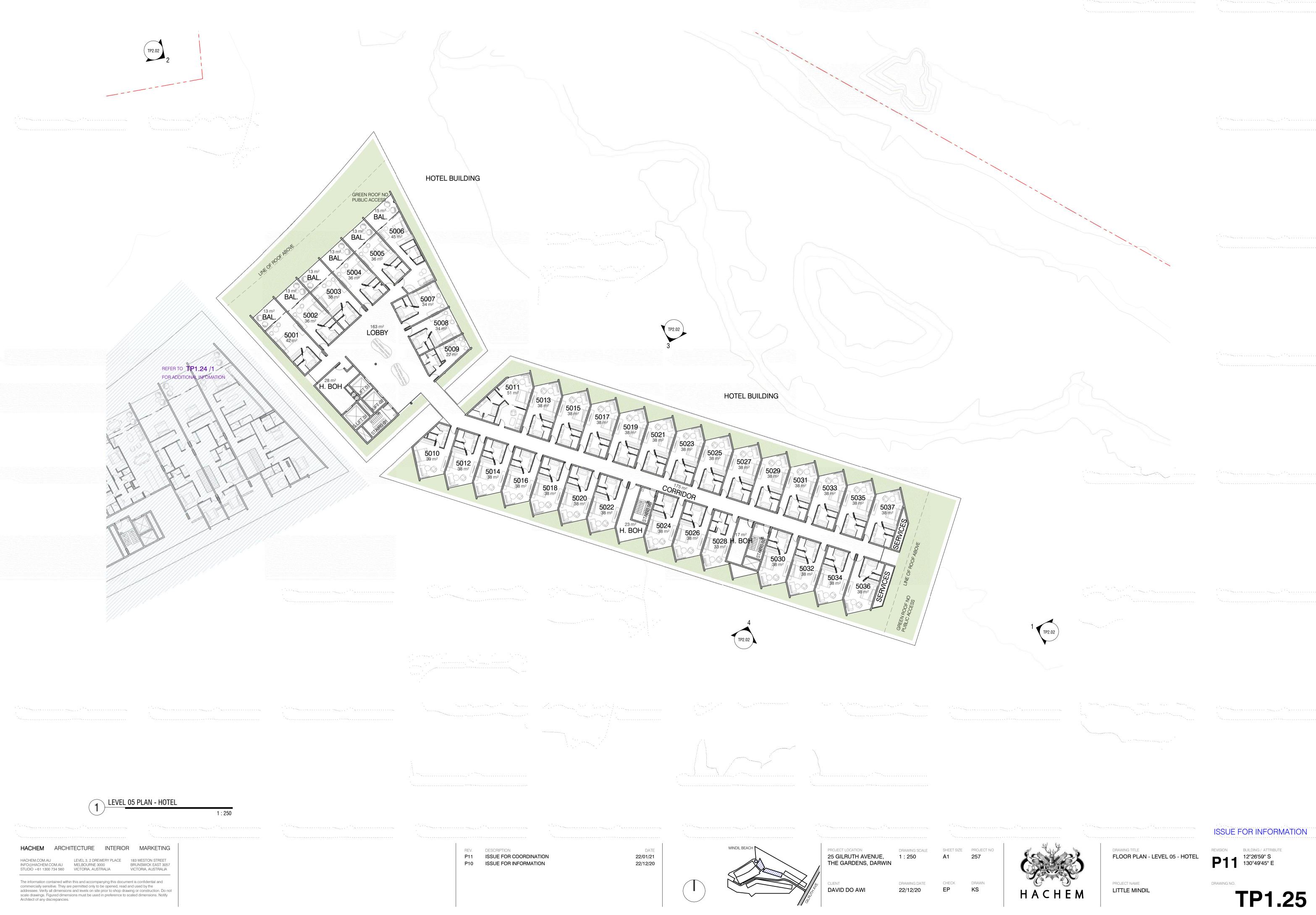
P11 12°26'59" S 130°49'45" E

PROJECT NAME LITTLE MINDIL

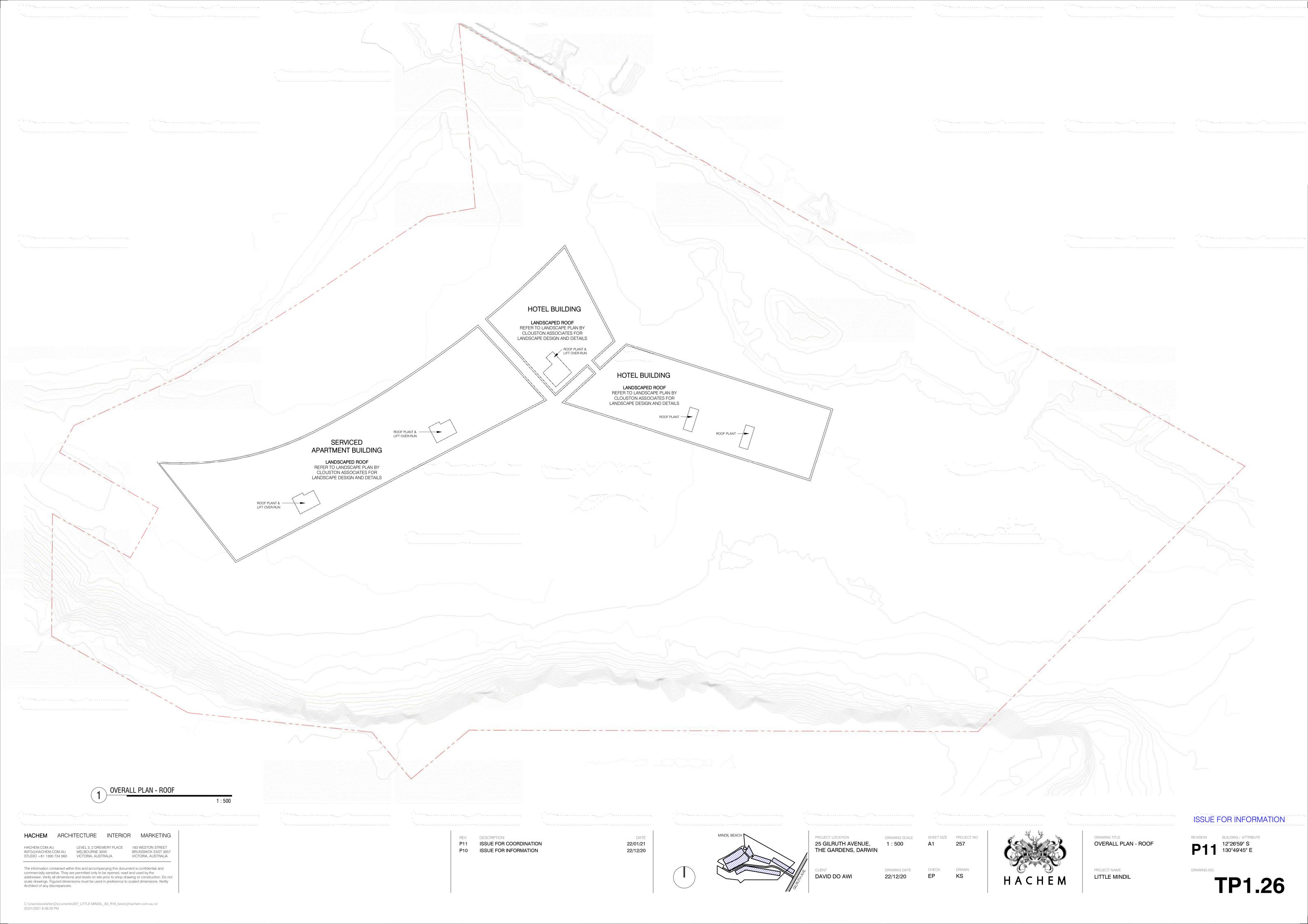
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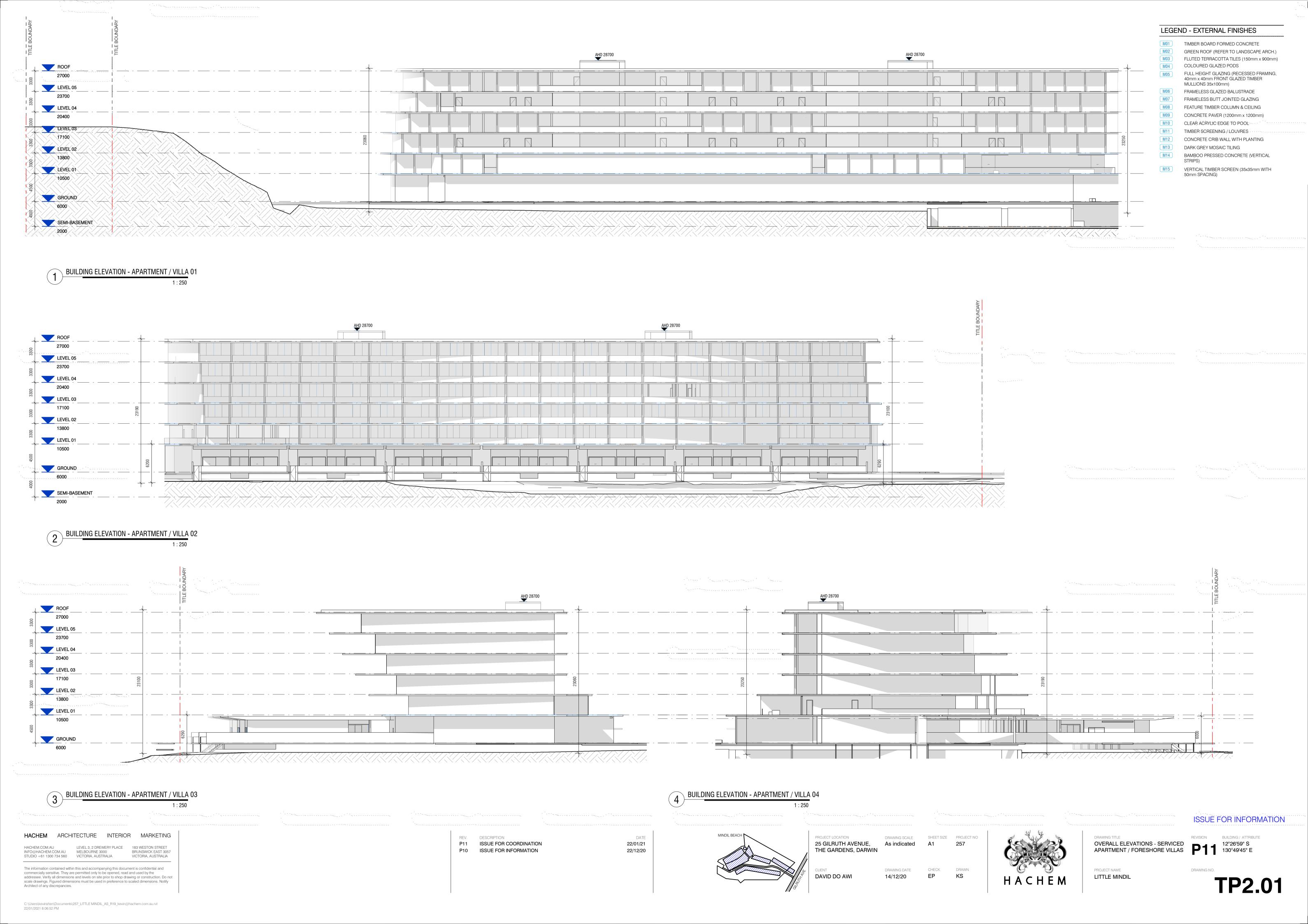


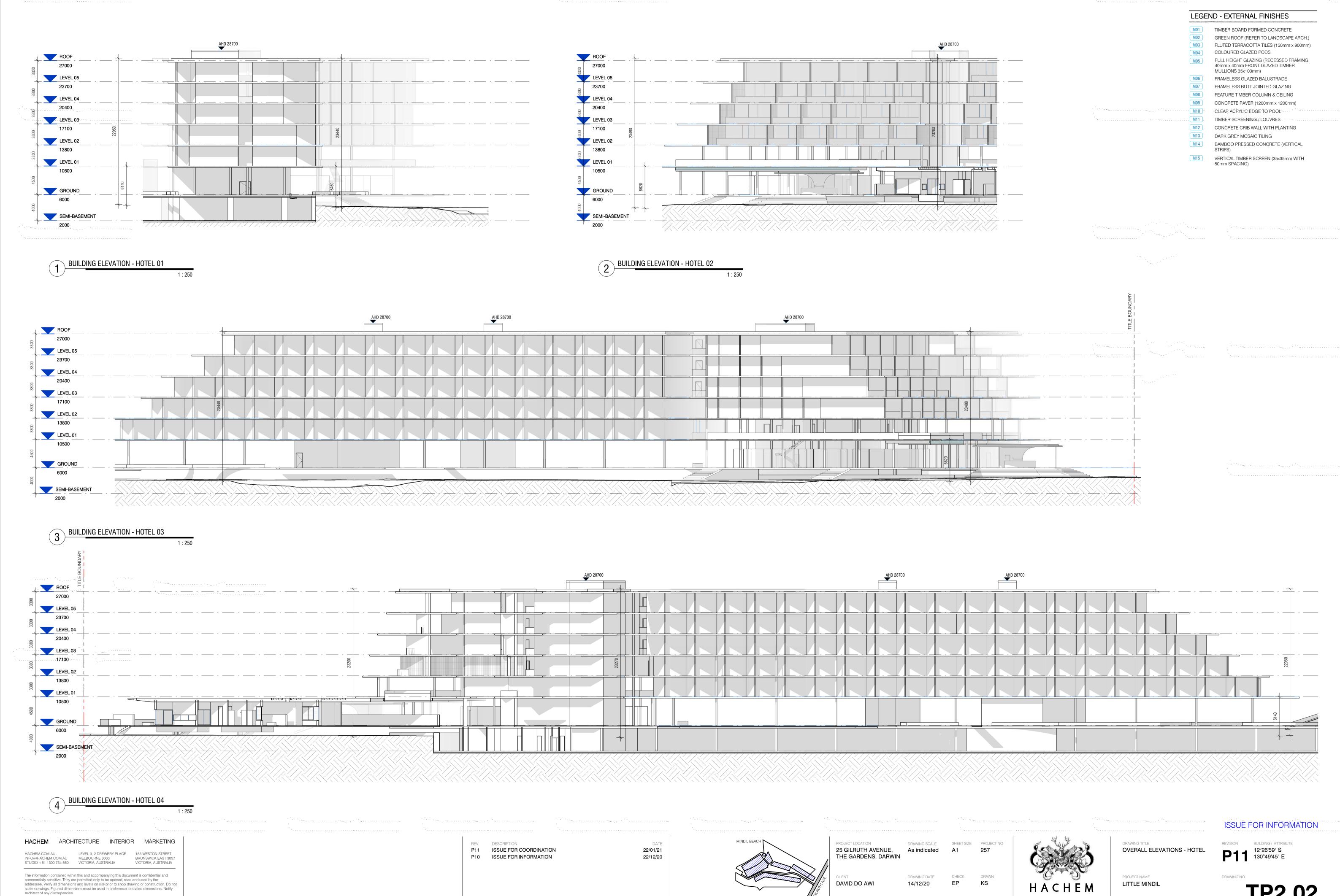




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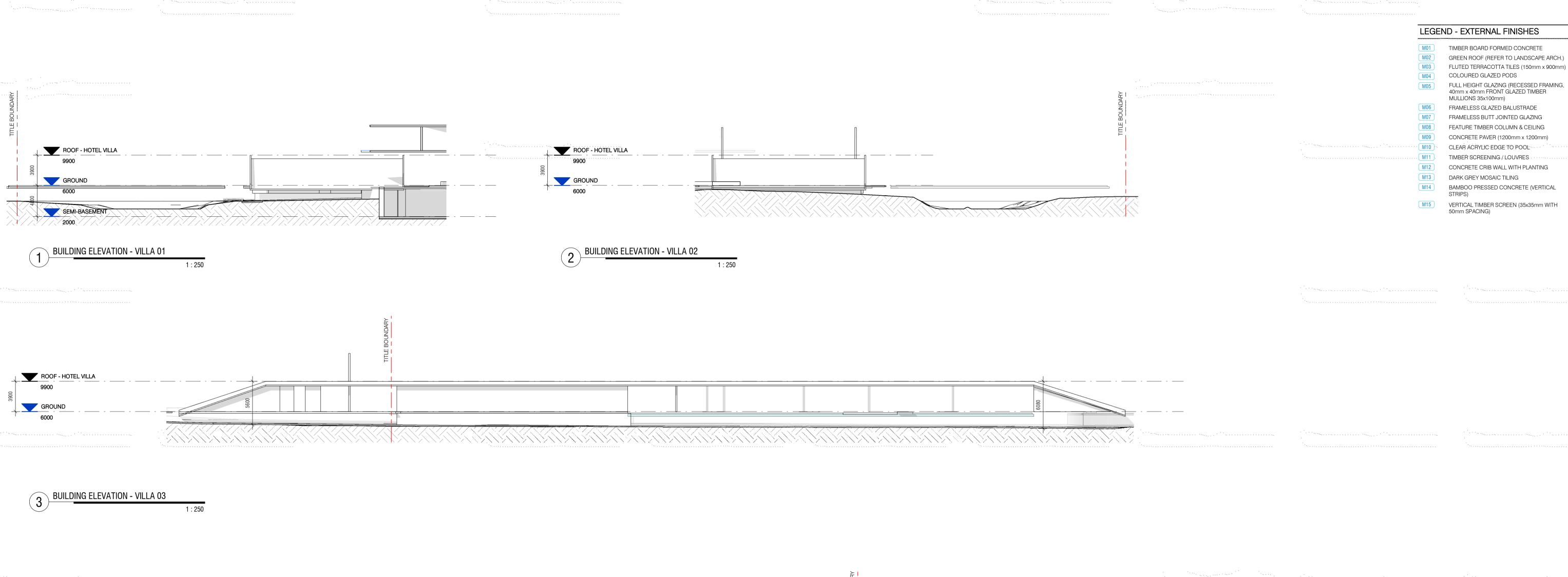


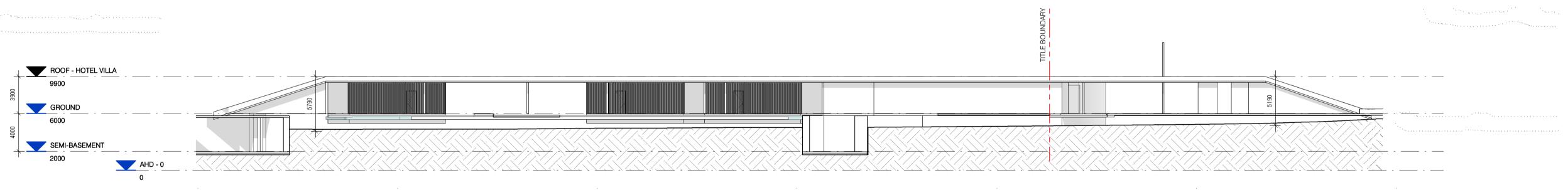




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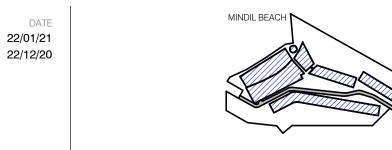
BUILDING ELEVATION - VILLA 04

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P11 ISSUE FOR COORDINATION
P10 ISSUE FOR INFORMATION





PROJECT LOCATION

25 GILRUTH AVENUE,
THE GARDENS, DARWIN

DRAWING SCALE

As indicated
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HACHEM

OVERALL ELEVATIONS - GARDEN VILLAS

PROJECT NAME

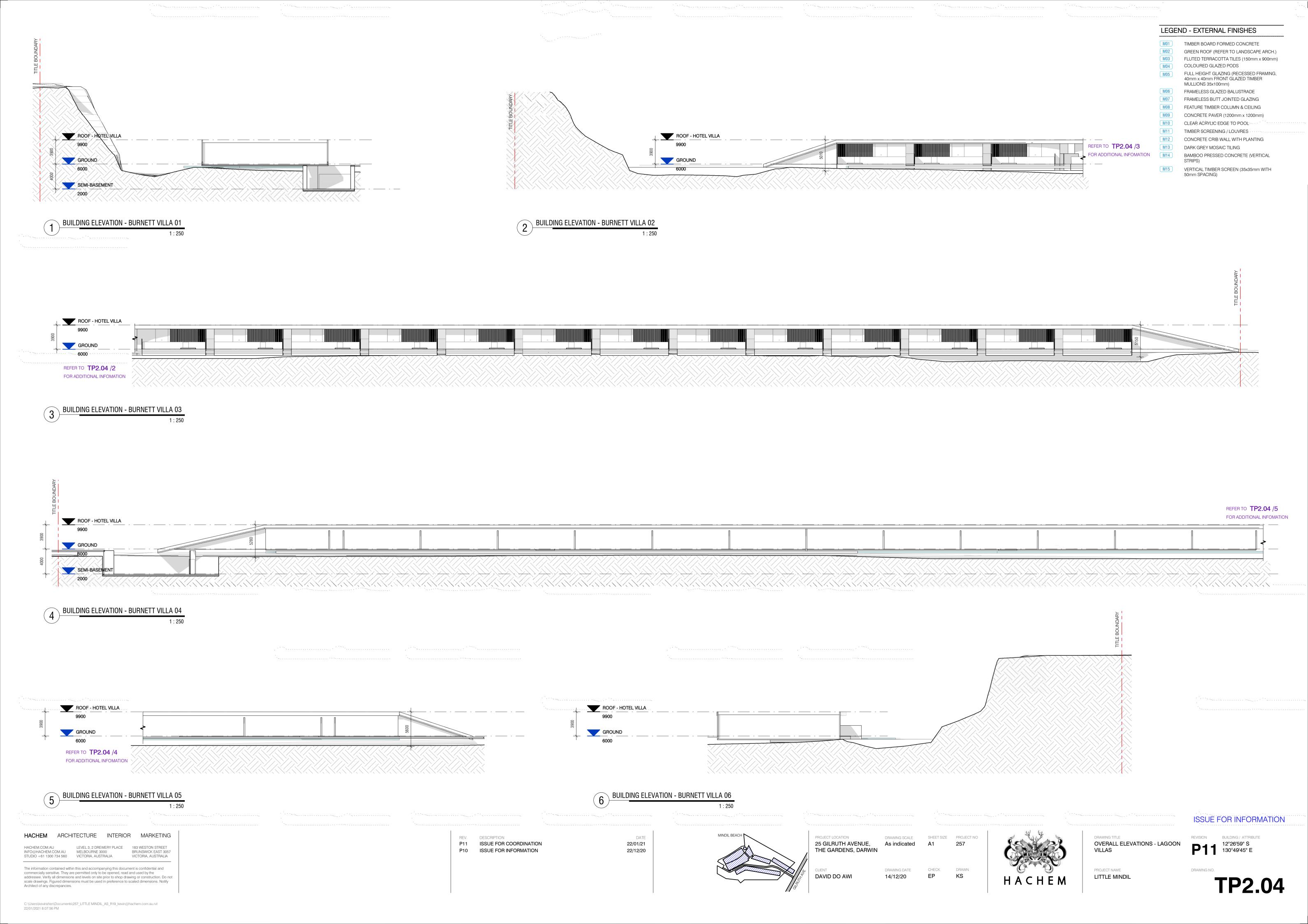
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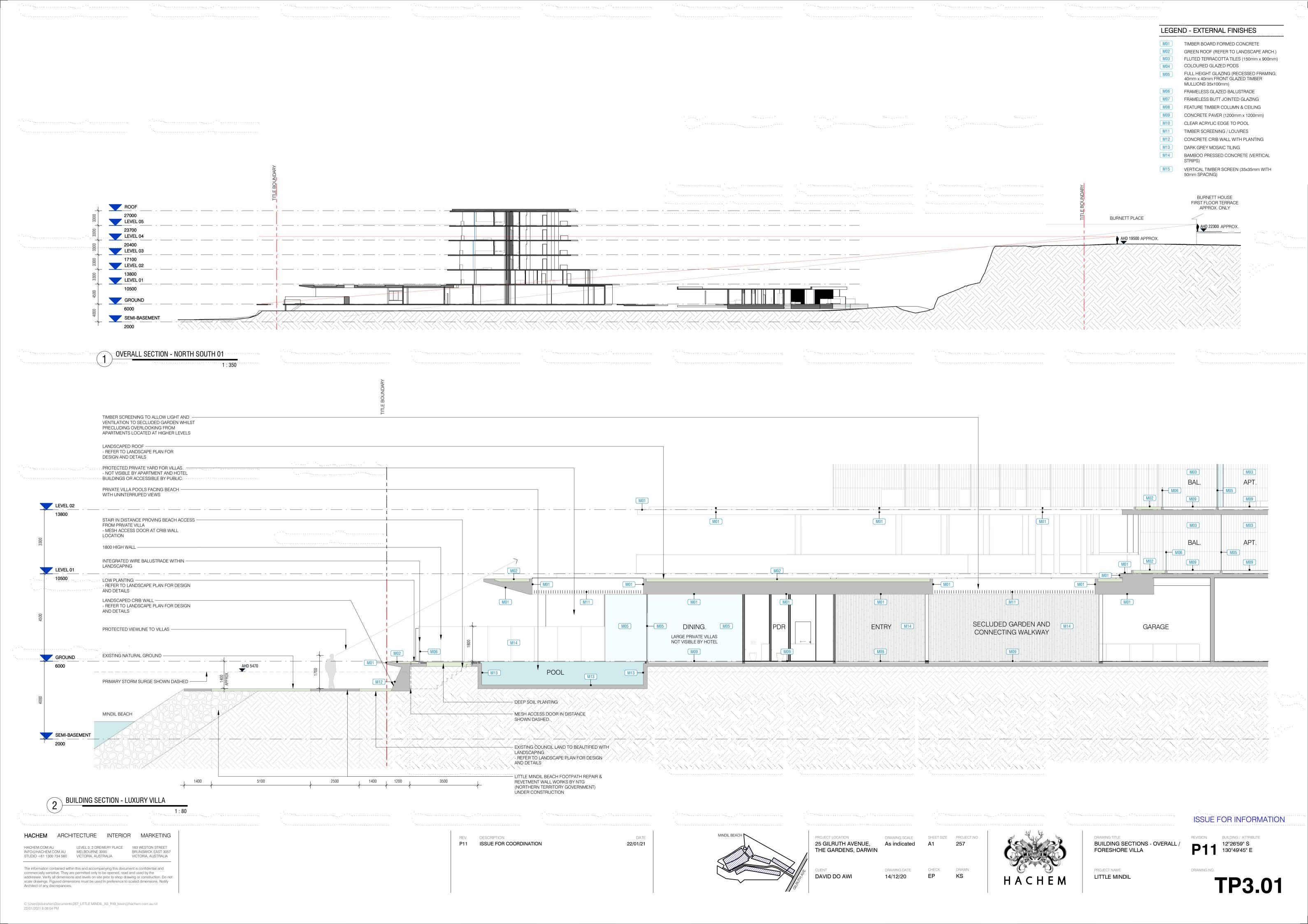
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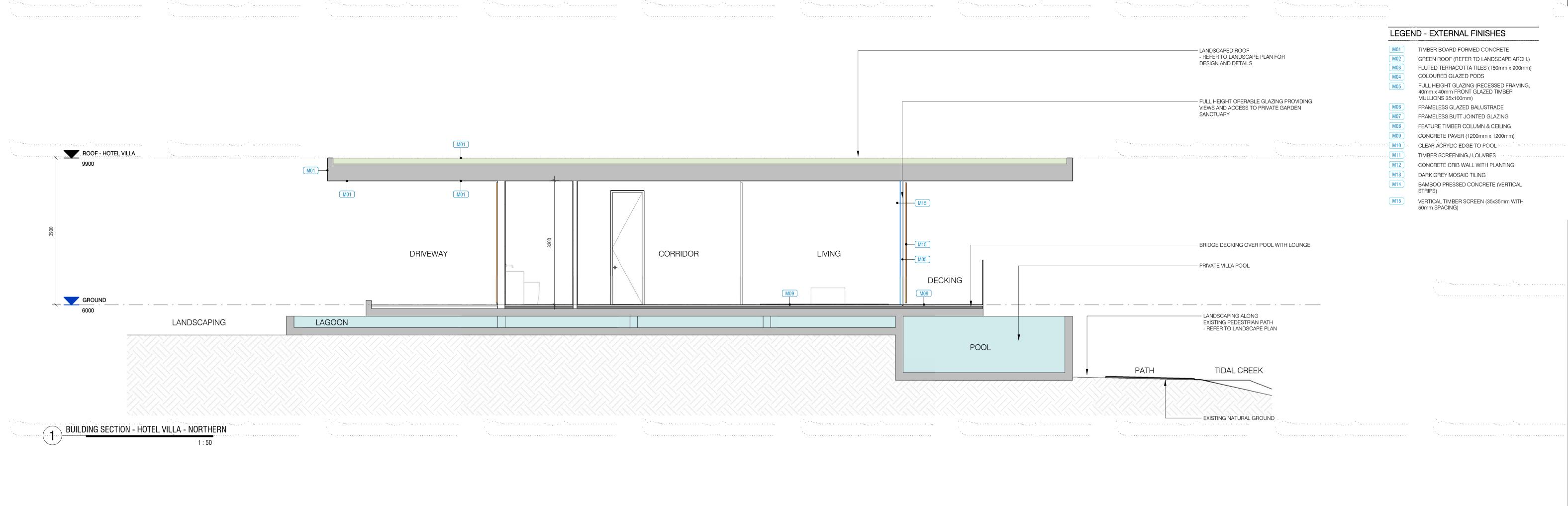
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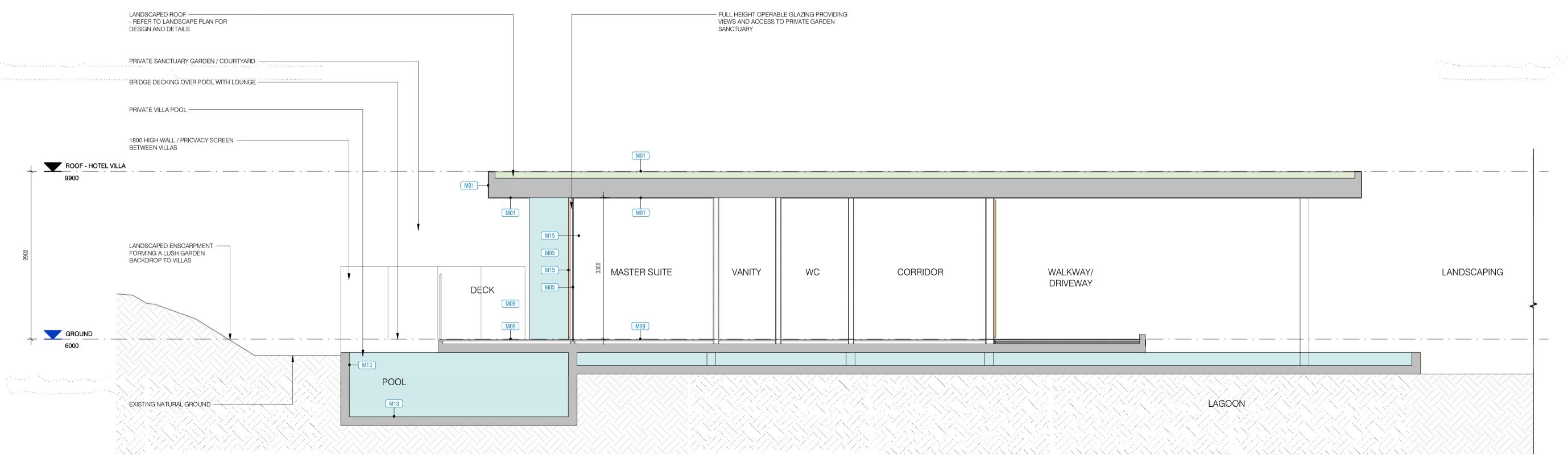
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TP2.03









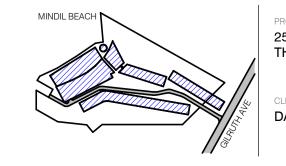
BUILDING SECTION - HOTEL VILLA - SOUTHERN

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P11 ISSUE FOR COORDINATION 22/01/21



PROJECT LOCATION

25 GILRUTH AVENUE,
THE GARDENS, DARWIN

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DRAWING SCALE
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PROJECT NO
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DRAWING TITLE
BUILDING SECTIONS - GARDEN &
LAGOON VILLA

PROJECT NAME

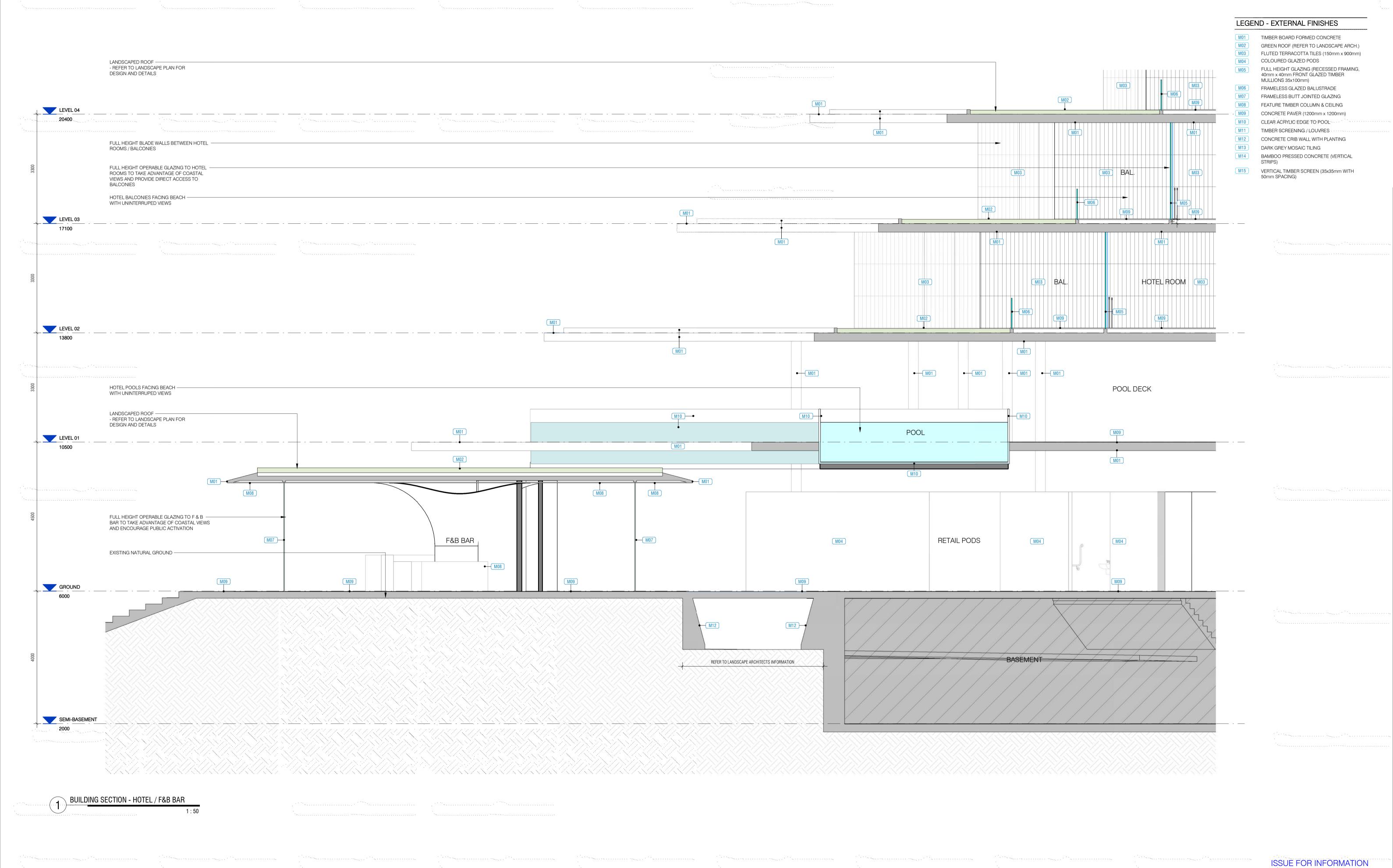
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REVISION BUILDING / ATTRIBUTE

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TP3.02

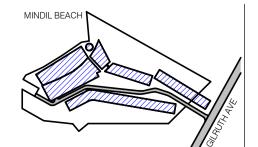


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REV. DESCRIPTION DATE
P11 ISSUE FOR COORDINATION 22/01/21



PROJECT LOCATION
25 GILRUTH AVENUE,
THE GARDENS, DARWIN

CLIENT
DAVID DO AWI

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As indicated
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PROJECT NO
257

DRAWING DATE
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14/01/20
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BUILDING SECTION - HOTEL / F&B BAR

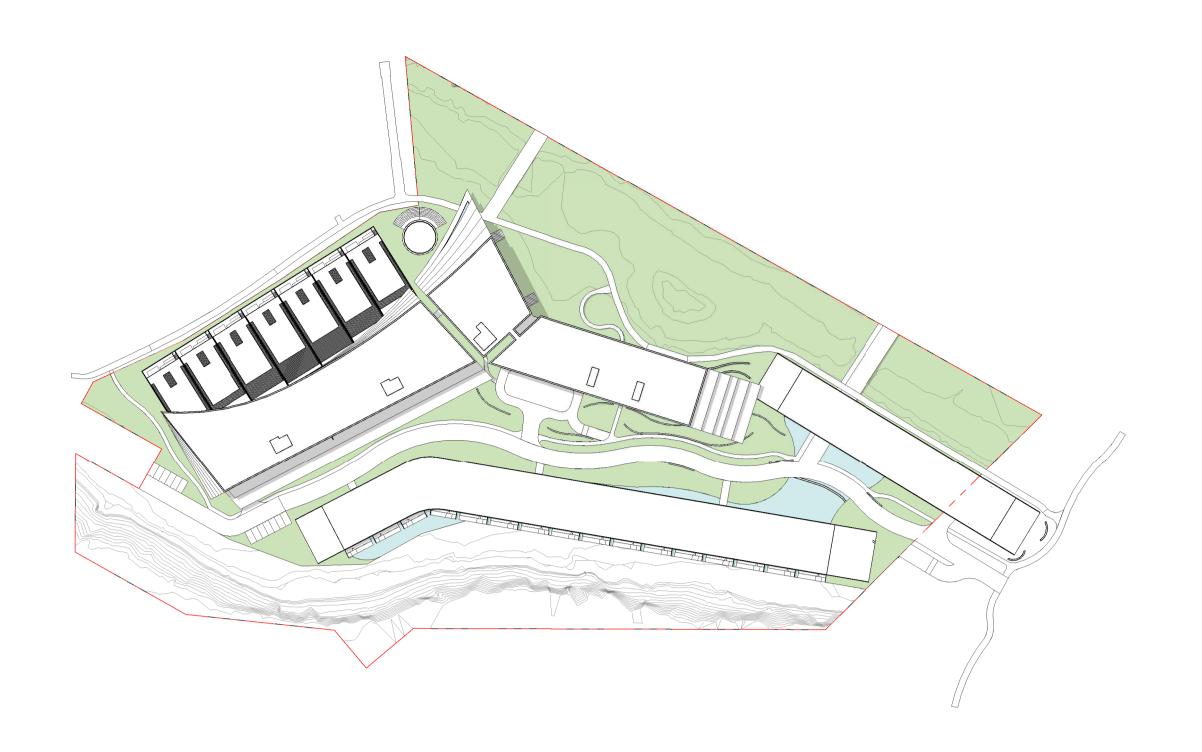
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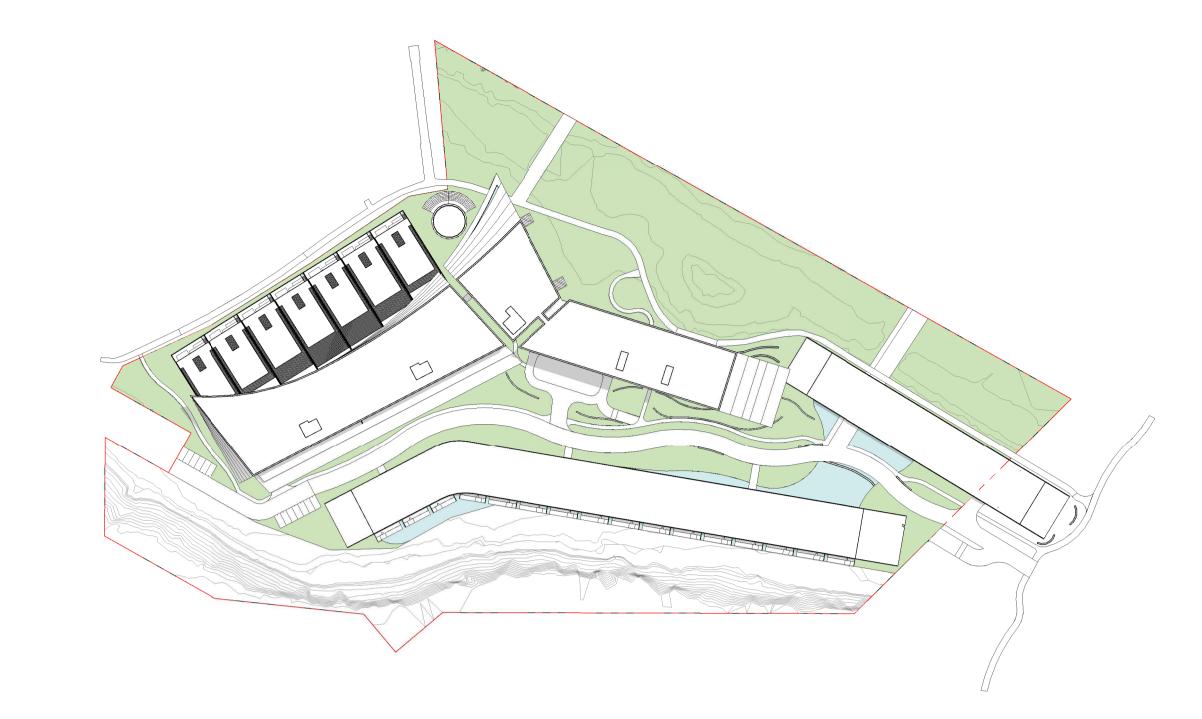
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REVISION BUILDING / ATTRIBUTE

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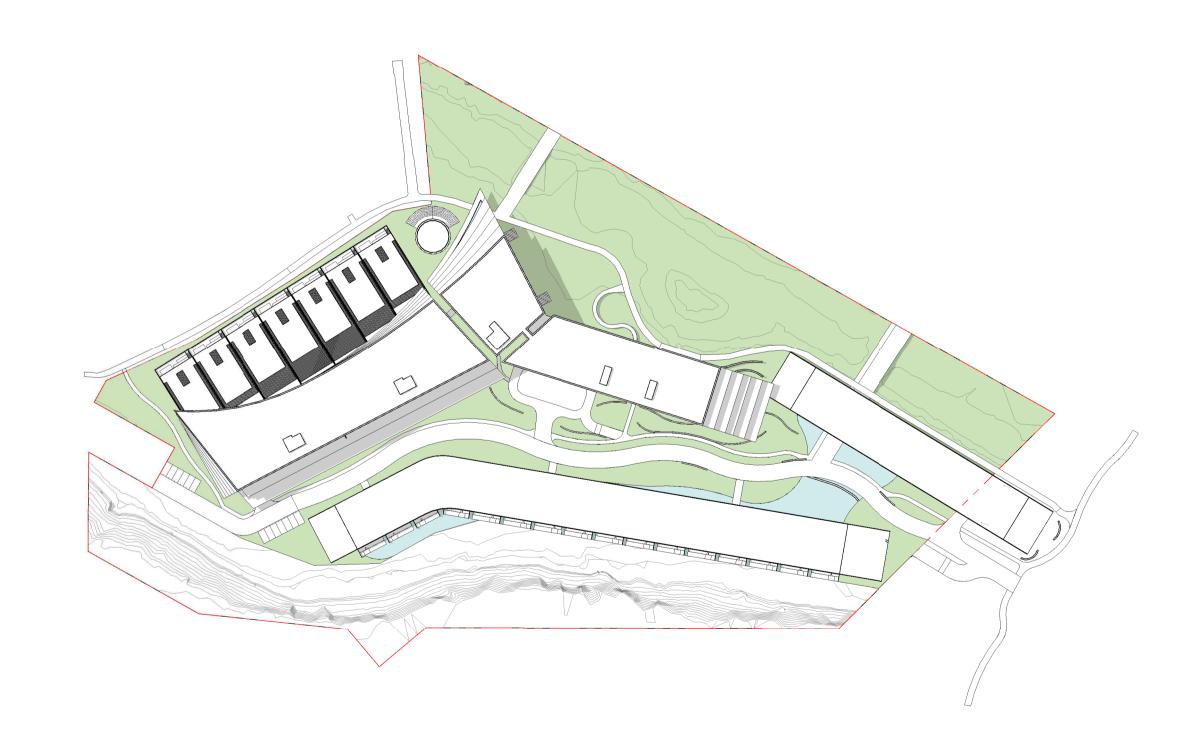
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1 SHADOW DIAGRAM - PROPOSED - SEP 22 9A

2 SHADOW DIAGRAM - PROPOSED - SEP 22 1 2PM



SHADOW DIAGRAM - PROPOSED - SEP 22 3PM 1:1500

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LEVEL 3, 2 DREWERY PLACE
INFO@HACHEM.COM.AU

MELBOURNE 3000
BRUNSWICK EAST 3057
STUDIO +61 1300 734 560
VICTORIA, AUSTRALIA
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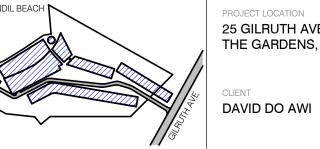
P11 ISSUE FOR COORDINATION
P10 ISSUE FOR INFORMATION

MINDIL BEACH

DATE

22/01/21

22/12/20



PROJECT LOCATION
25 GILRUTH AVENUE,
THE GARDENS, DARWIN

CLIENT
DAVID DO AWI

DRAWING SCALE
1: 1500
A1
257

CHECK
DRAWN
DAVID DO AWI

DRAWING DATE
14/12/20
EP
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DRAWING TITLE
PROPOSED SHADOW DIAGRAMS

PROJECT NAME

LITTLE MINDIL

BUILDING / ATTRIBUTE
12°26'59" S
130°49'45" E

TP4.01

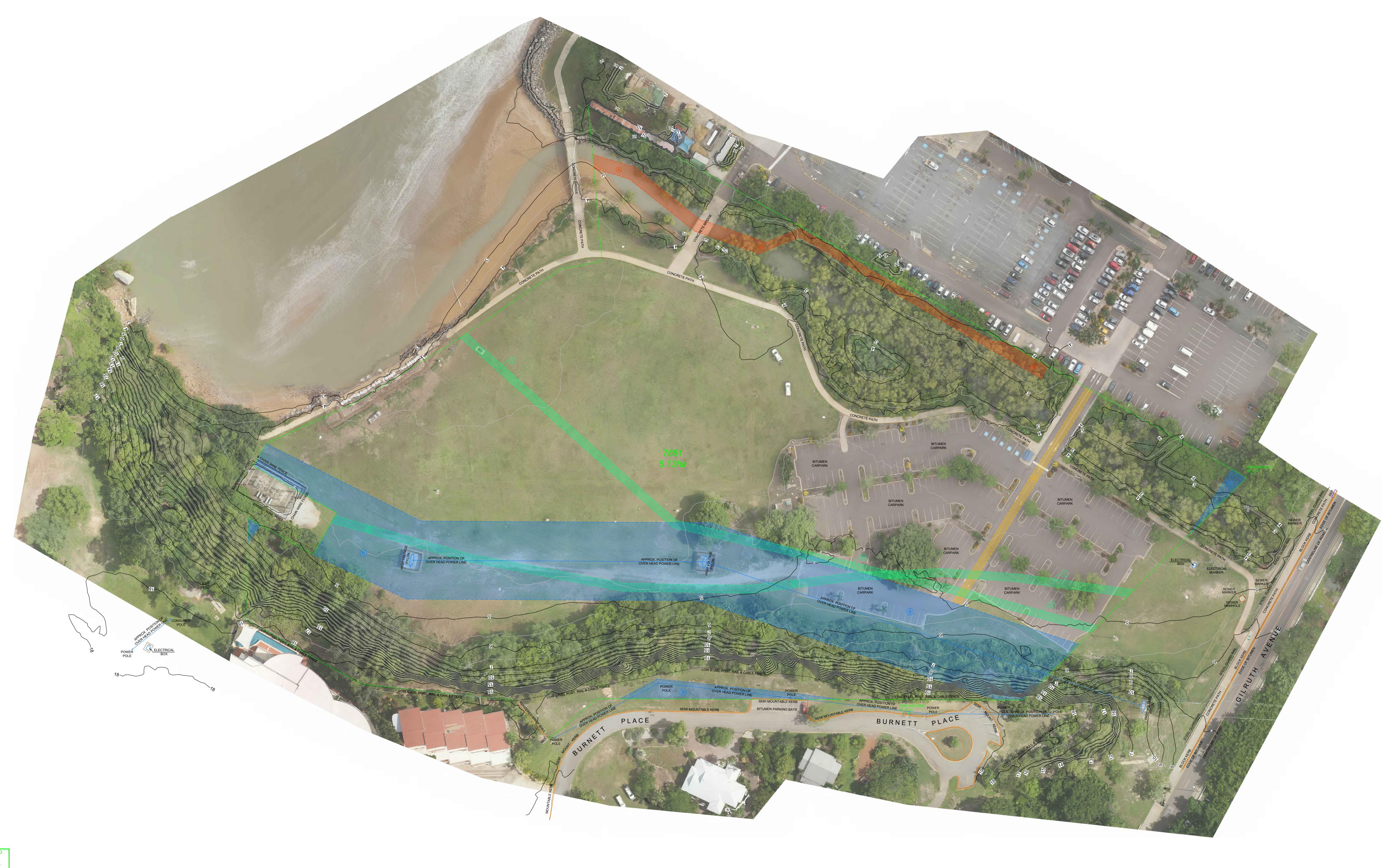
ISSUE FOR INFORMATION



Ref: 23085 C R001 REV01 01.02.2021.docx 01 February 2021

> Appendix B Site Survey

STAY WIRE ANCHOR GAS METER GAS VALVE



BOUNDARY INFORMATION SOURCED FROM THE NORTHERN TERRITORY ATLAS & SPATIAL DATA DIRECTORY.
BOUNDARY LINE WORK HAS BEEN
CONVERTED TO MGA BASED ON CRM
S95220043, WITH A SCALE FACTOR OF 1.00008948 AND A GRID CONVERGENCE OF 0°23'40.83" BEARINGS & DISTANCES SHOWN ARE FROM SURVEY PLAN S2009/255A

A = EASEMENT (SEWERAGE)
BENEFIT TO THE POWER &
WATER CORPORATION.

MAJOR CONTOURS 00 -MINOR CONTOURS B = EASEMENT (ELECTRICITY SUPPLY

BENEFIT TO THE POWER & WATER CORPORATION.

C) = EASEMENT (ELECTRONIC COMMUNICATIONS SUPPLY) BENEFIT TO TELSTRA

CORPORATION LTD.

D = EASEMENT (RIGHT OF WAY)
BENEFIT TO THE POWER &
WATER CORPORATION.

NOTE: THIS PLAN HAS BEEN PREPARED WITH 3D VALUES AND HAS MGA94 ZONE 52 COORDINATES.

CONTOUR LEGEND - 0.5m INTERVALS

NOTE: AHD LEVEL DERIVED FROM COORDINATED REFERENCE MARK S95220043 - RL = 19.267
 SURVEY CONTROL COORDINATE LISTING MGA94 ZONE 52

 NAME
 EASTING
 NORTHING
 ELEVATION

 CRM S09255001
 699038.415
 8623033.555
 4.130

 CRM S95220043
 698906.915
 8622941.908
 19.267

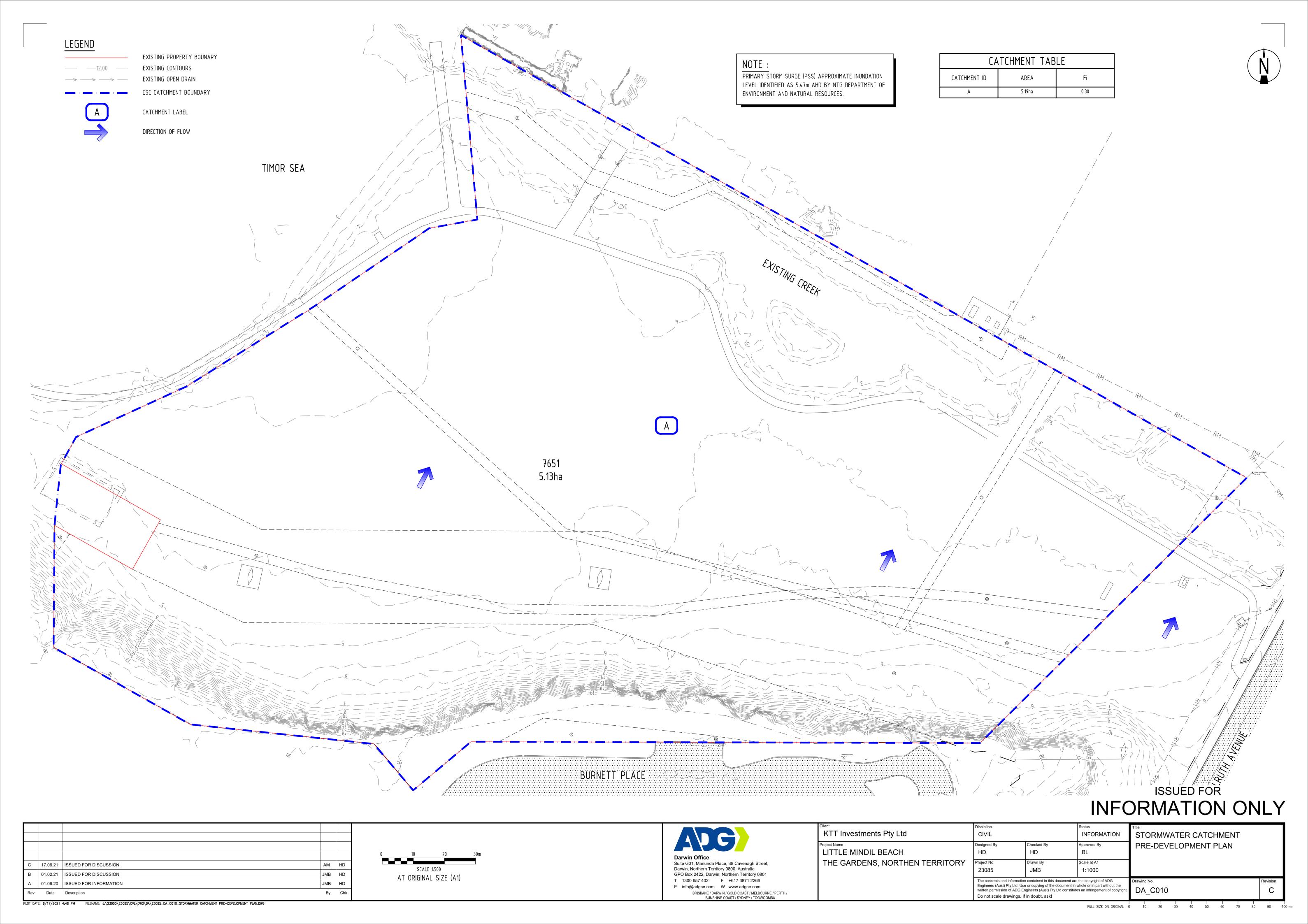
					LOT : 7651 (# 25) GILRUTH AVENUE		CLIENT : KTT INVESTM
					SUBURB : THE GARDENS	SURVEY PLAN: S2009/255A	SURVEYED ON: 14/02/2020
					AUTHORITY : CITY OF DARWIN	C/T : 832 / 126	SURVEYOR : LV/BZ
					BUILDERS / CLIENTS: SITE SURVEY only. The information shown on this plan is current at the time of survey. Verify boundary information, easements etc. from the Certificate of Title, Plan / Diagram or a Boundary Repeg. Boundary position approximate only. Location of boundary pegs or fences in relation to the boundary lines are not guaranteed. Sewer / water may vary from schematic presentation, clearances to be checked on site. Confirm service information with relevant Authorities. Refer to "Dial Before You Dig" for underground service confirmation. Consult Land Surveys on any anomaly before design and construction.		Land Sur
REV	DESCRIPTION	DRN	DATE	APP	SCALE: 10 0 5 1:500 @ A0 Landscape	10 20 30	JOB No : PLAN: 2000201 - FS

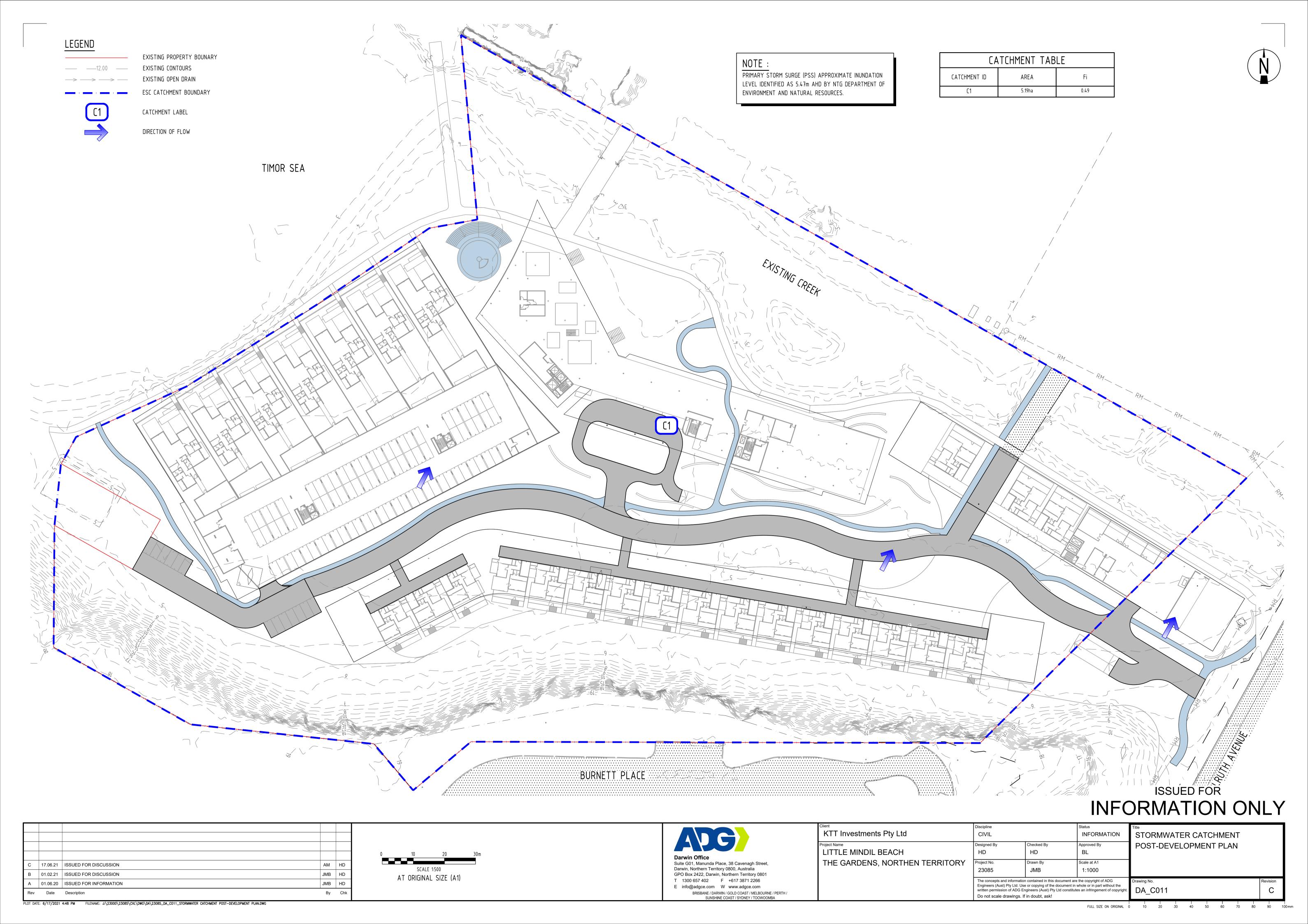
: KTT INVESTMENTS PTY. LTD.

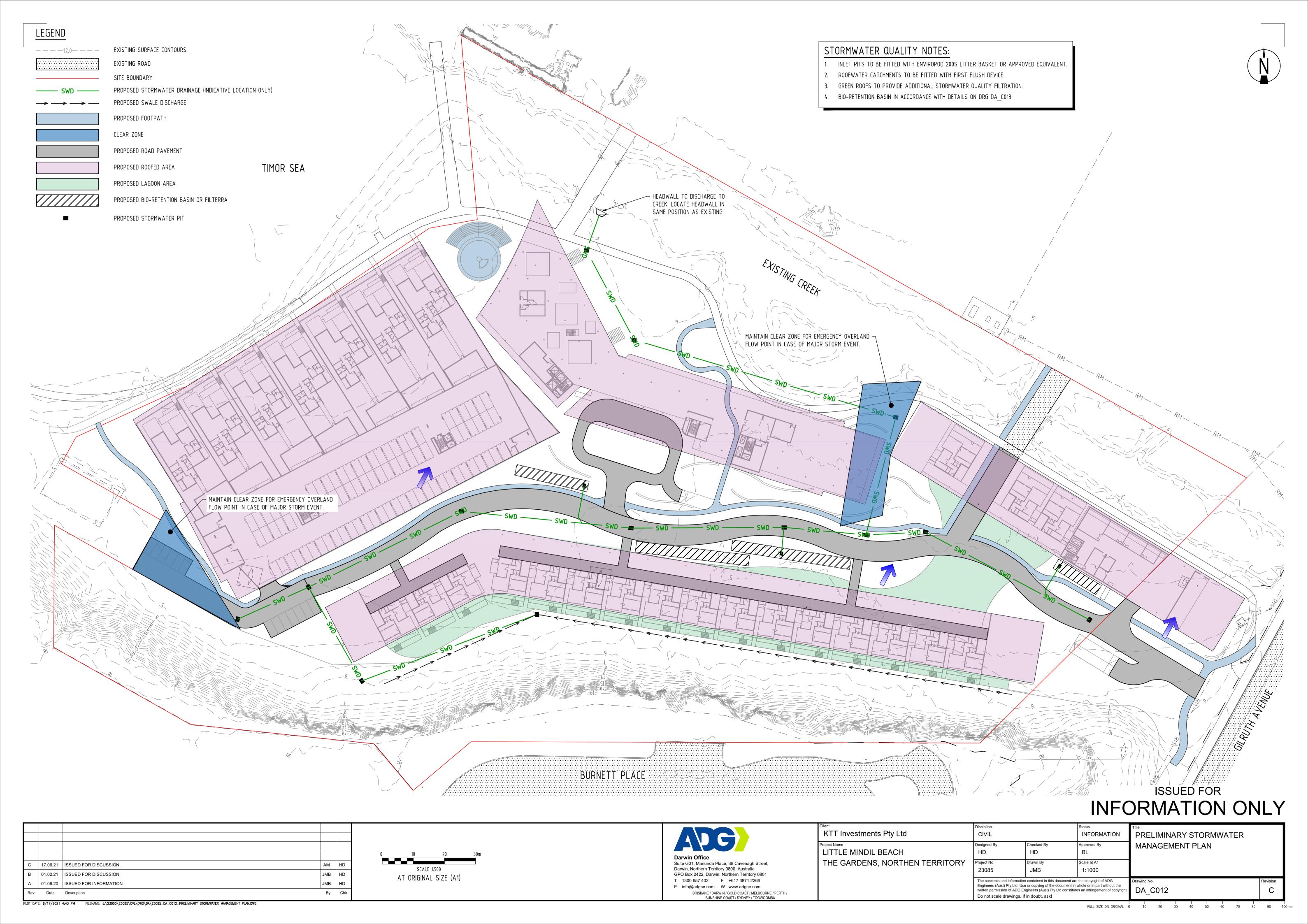




Appendix C ADG Preliminary Plans







BIO RETENTION NOTES:

INSPECTIONS:

1. CONTRACTOR TO CONTACT ADG ENGINEERS AT LEAST 48 HOURS PRIOR TO PLACEMENT OF EACH LAYER TO ORGANISE INSPECTION. EACH LAYER IS NOT TO BE PLACED UNTIL WRITTEN CONFIRMATION PROVIDED BY ADG ENGINEERS.

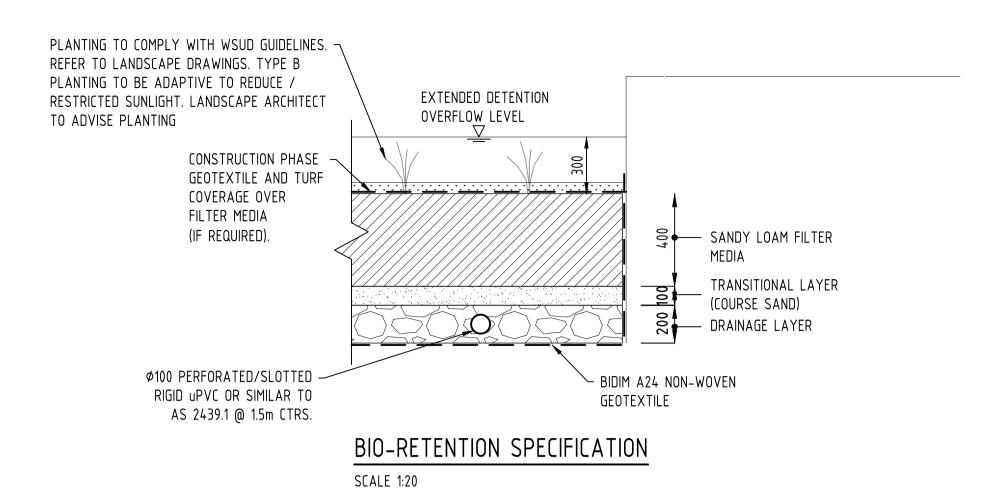
FILTRATION LAYER:

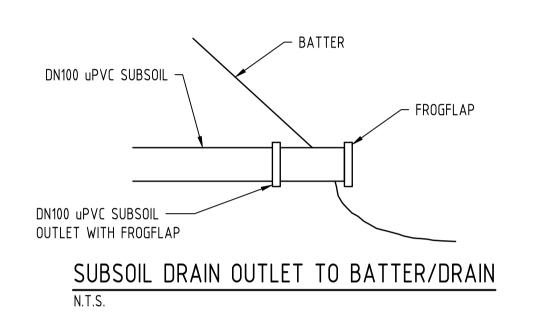
- 1. CONTRACTOR TO PROVIDE ADG ENGINEERS WITH TEST INFORMATION OF PROPOSED FILTER MEDIA TO CONFIRM THE BELOW PARAMETERS.
- 2. MATERIAL TO CONSIST OF SANDY LOAM OR EQUVALENT MATERIAL. 3. MATERIAL TO HAVE 5% - 10% ORGANIC CONTENT IN ACCORDANCE WITH AS1289.4.1.1
- 4. MATERIAL TO HAVE AN AVERAGE PARTICAL SIZE (D50) OF 0.45mm.
- 5. SATURATED HYDRAULIC CONDUCTIVITY TO BE BETWEEN 100-300mm/HR DETERMINED IN ACCORDANCE WITH AS 4419-1998 APPENDIX H SOIL PERMEABILITY.
- 6. pH BETWEEN 6 & 7.
- 7. TN CONTENT OF FILTER MEDIA TO BE <400mg/kg 8. ORTHOPHOSPHATE CONTENT TO BE <50mg/kg
- DRAINAGE LAYER:
- 1. MATERIAL TO CONSIST OF 2-5mm GRAVEL.

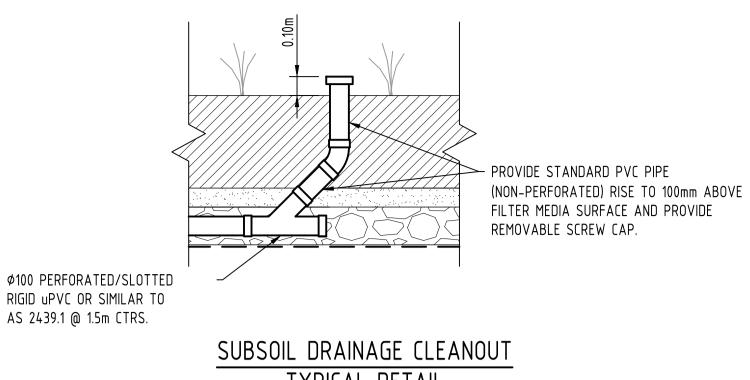
PERFORATED PIPE:

1. 100¢ SLOTTED RIGID uPVC OR SIMILAR TO AS 2439.1 OR APPROVED EQUIVALENT MIN 0.5% GRADE @ 1.5m CTRS

- CONSTRUCTION PHASE: 1. COVER FILTRATION LAYER IN GEOTXTILE, 50mm TOPSOIL & TURF STRIPS
- PERPENDICULAR TO FLOW. 2. GEOTEXTILE TO BE REMOVED ONLY WHEN UPSTREAM SEDIMENT LOADS ARE CONTROLLED.
- 3. BASIN TO BE PLANTED AS PER THE APPROVED LANDSCAPE PLANS

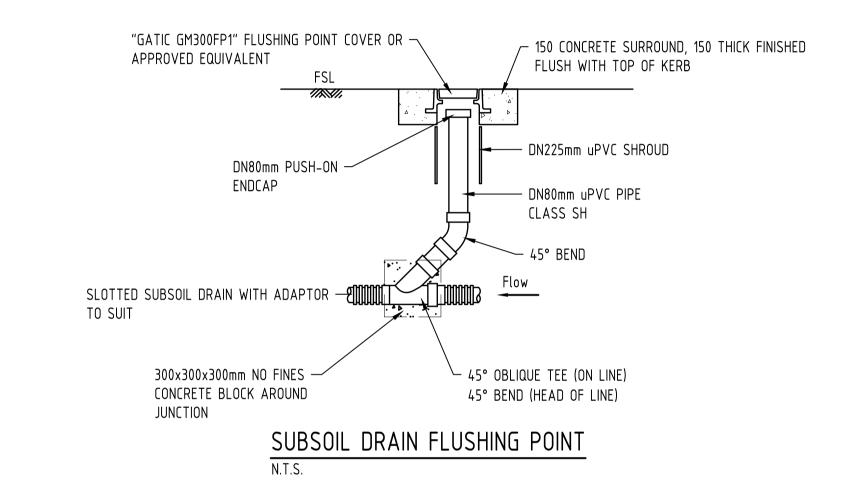


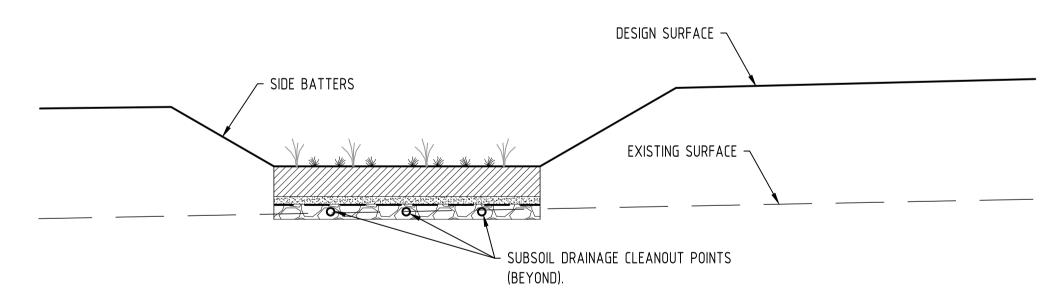




TYPICAL DETAIL (REFER STD IPWEAQ DETAIL ON DRG WSUD-003).

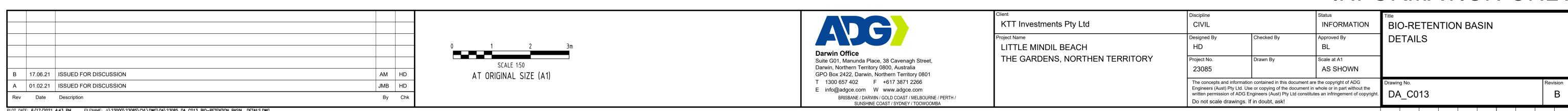
SCALE 1:20





TYPICAL BIO-RETENTION (TYPE A) SECTION

ISSUED FOR INFORMATION ONLY



PLOT DATE: 6/17/2021 4:43 PM FILENAME: J:\23000\23085\CVL\DWG\DA\23085_DA_C013_BIO-RETENTION BASIN DETAILS.DWG

FULL SIZE ON ORIGINAL 0 10 20 30 40 50 60 70 80 90 100mm



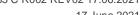




Appendix D Rational Method Calculations



Project Little Mindil E	Beach															Job # 23085			Rev
																Made by / HD	date	Apr-20	
Subject Pre-Develop	ment Catch	ment F	Runoff													Checked / SW	date	Apr-20	
					RA	TION	AL ME	THOD C	ALCUL	ATION	S (PRE-DE	EVELOI	PMENT)						
PRE-DEV CATCH- MENT ID	AREA (ha)	Fi	Impervious Area (ha)	Pervious Area (ha)	C10	C1	C100	CA100	Sheet Flow Length (m)	Slope	Mannings "n"	Sheet Flow Time (mins)	Channel Flow Time (mins)	Time of Conc. "tc" (mins)	Q1 Rainfall Intensity (mm/hr)	Q ₁₀₀ Rainfall Intensity (mm/hr)	Q1 Peak Flow (m3/s)	Q10 Peak Flow (m3/s)	Q100 Peak Flow (m3/s)
EX1	5.190	0.30	1.557	3.633	0.76	0.61	0.912	4.733	150	1.33	0.035	19	0	18.8	89.7	217.8	0.786	1.641	2.864
Project		4) Time	esity Frequency ne of concentra O value for Fi va	ntion is sum o	of overla	nd shee	et flow tim	e (Friend's	eq.) plus c	hannel tir	ne (Argue me	thod)	tem'			Job#			Rev
Little Mindil E	Beach															23085 Made by / HD		Anr. 20	(
																ПП		Apr-20	
Subject Post-Develo	pment Catc	hment	Runoff													Checked / SW	date	Apr-20	
,	pment Catc	hment	Runoff		RA	TION	AL MET	THOD C	ALCULA	TIONS	(POST-D	EVELO	PMENT)				date	Apr-20	
POST-DEV		hment Fi	Runoff Impervious Area	Pervious Area	RA C10	TION/	AL ME 7	CA100	Sheet Flow	TIONS Slope	Mannings	Sheet Flow	Channel Flow	Time of Conc.	Q1 Rainfall Intensity	Q100 Rainfall	Q1 Peak	Q10 Peak	Q100 Peak Flow
Post-Develo			Impervious						Sheet		Mannings	Sheet	Channel			Q100	Q1	Q10	
POST-DEV CATCH-	_ AREA (ha)		Impervious Area	Area	C10				Sheet Flow Length	Slope	Mannings "n"	Sheet Flow Time (mins)	Channel Flow Time (mins)	Conc. "tc"	Rainfall Intensity	Q100 Rainfall Intensity	Q1 Peak Flow	Q10 Peak Flow	Peak Flow







Appendix E Storm Surge Flood Mapping and NTG Correspondence

Request from Harris Davidson about Storm Surge Level for the property Parcel – 7651, Town of Darwin, NT

The Surface Water group in DENR has carried out desktop study on storm surge level and flooding levels query for the property (**Parcel 7651, Town of Darwin, NT**) and have made the following comments based on available (published) relevant Mappings such as Storm Surge Flood Mappings and digital data as GIS layer:

SURFACE WATER COMMENTS:

• The lot of interest (**Parcel 7651** as shown in Figure 1 below) is affected by both Primary Storm Surge (PSS) and Secondary Storm Surge (SSS) inundation / flooding of 100 year and 1000 year ARI with approximate inundation levels as shown in Table 1 below (*Source: Darwin Area Storm Surge Inundation for 2100, November 2014 / prepared by GHD Pty Ltd for DLRM*).







Figure 1 Location of Primary and Secondary Storm Surge Levels (P – Primary; S – Secondary)

Table 1 Primary and Secondary Storm Surge Levels

Locations	Primary Storm	Secondary Storm
	Surge Level (PSS)	Surge Level (SSS)
	(mAHD)	(mAHD)
P1	5.46	-
P2	5.46	-
Р3	5.47	-
P4	5.47	-
P5	5.47	-
P6	5.46	-
S1	-	5.75
S2	-	5.86
S3	-	5.85
S4	-	5.79
S5	-	5.75
S6	-	5.76

This document contains information obtained through a desktop assessment. DENR has made every reasonable effort to provide current and accurate information, but it does not make any guarantees regarding the accuracy or completeness of the information. The information in this document does not constitute professional advice and should not be relied upon. You should obtain your own professional advice.





Appendix F MUSIC Modelling Results



MUSIC Model Information

Introduction:

The quality of stormwater runoff and the impact of the proposed stormwater quality improvement measures were analyzed using MUSIC Version 6.3.0 according to the *MUSIC Modeling Guidelines Version 3.0, Water by Design 2018.* The source nodes in the model are split into various types and a summary of the area breakdown is presented below:

Meteorological Data:

The MUSIC model was carried out using the following parameters:

- Modeling period should be 10 years with a time step of 6 minutes
- The nearest available 6 minute time step rainfall series to the subject site is Darwin, with a mean annual rainfall of 1728 mm, and data from: 1987 to 1996.

Evaporation was applied as monthly mean. The mean annual evaporation was 1427 mm.

Source Nodes, Fractions Impervious:

The areas of the source nodes were estimated from the Functional Layout Plan as shown in Appendix C.

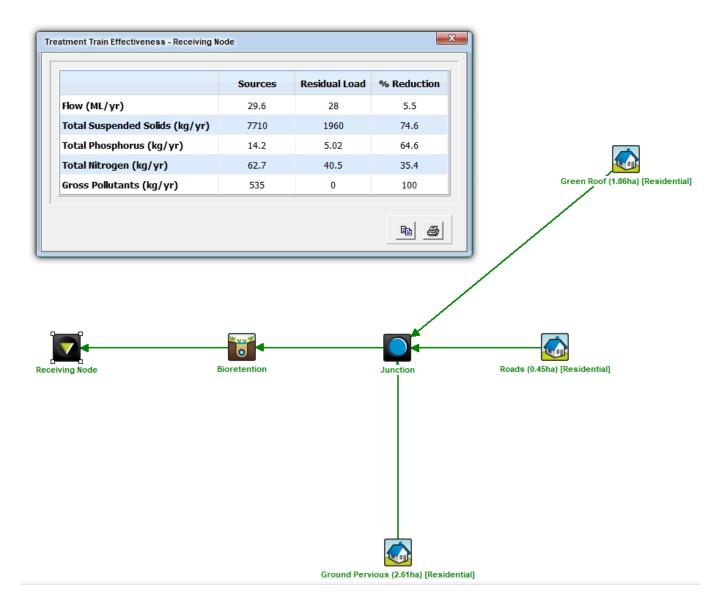
Source Nodes - Pollutant Exports:

Rainfall runoff and pollutant export parameters were assigned per **Tables 3.7** and **3.8** of the Water by Design MUSIC Modeling Guidelines Version 1.0 (2010).

The rainfall runoff and pollutant export parameters for a residential development were adopted.

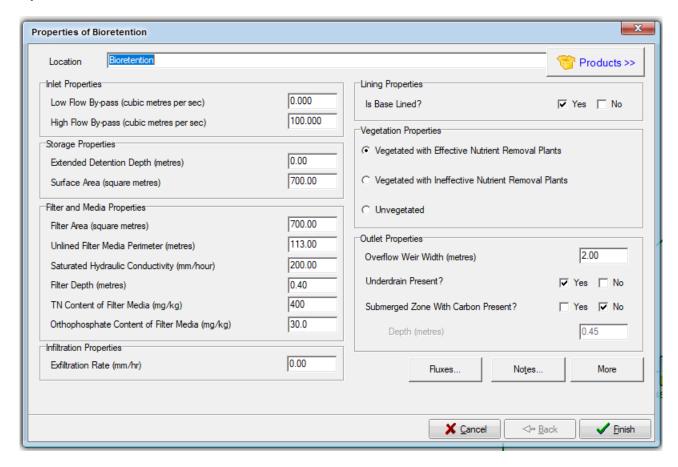


Option 1 Treatment Train Diagrams & Results



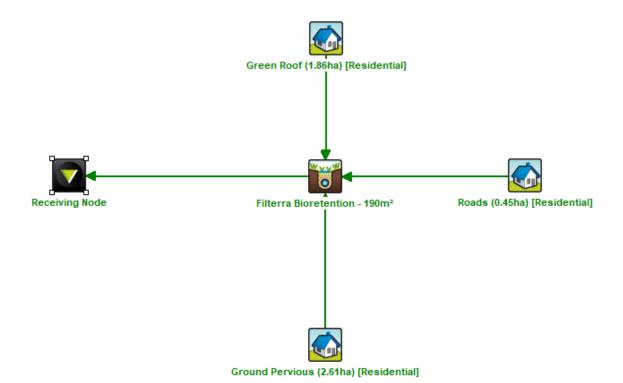


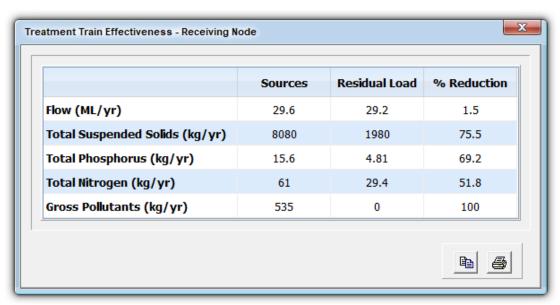
Option 1 Bio-retention Basin:





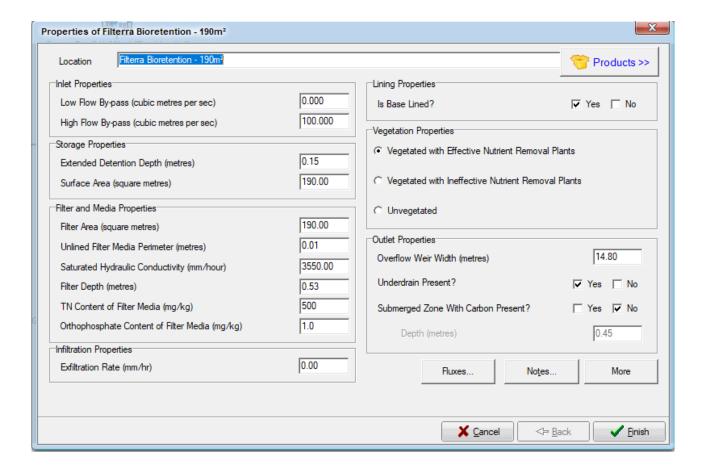
Option 2 Treatment Train Diagrams & Results







Option 2 Filterra:



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